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Influence of Block and Mortar Strength on Shear Resistance of Concrete Block Masonry Walls

Kyle Woodward
Frank Rankin

U.S. DEPARTMENT OF COMMERCE
National Bureau of Standards
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U.S. DEPARTMENT OF COMMERCE, Malcolm Baldrige, *Secretary*
NATIONAL BUREAU OF STANDARDS, Ernest Ambler, *Director*

ABSTRACT

Data from seventeen masonry wall panel tests are presented. All of the walls are ungrouted, unreinforced, and constructed with hollow concrete block. The primary variables in the test series are block and mortar strength, but the applied vertical compressive stress and wall aspect ratio are also varied. The walls are built with either a "high" strength block or a "low" strength block having gross area unit strengths of approximately 1800 psi and 1300 psi, respectively. The mortar is either a Type S or a Type N mortar and, for convenience, is identified as high and low strength mortar, respectively. Thirteen of the wall panels have nominal dimensions of 64 in. long x 64 in. high x 8 in. thick, but two of the walls are 96 in. long and the remaining two walls are 48 in. long. The applied net area vertical compressive stress is constant for a given test, but varies between 100 psi and 400 psi for tests in the series reported herein. The walls are tested in the NBS Tri-directional Test Facility using fixed-ended boundary conditions at the top and bottom of the walls. A vertical compressive stress is applied and maintained while in-plane lateral displacements are imposed at the top of the wall. The test results indicate that for the lower levels of vertical compressive stress the effect of block and mortar strength on maximum in-plane shear resistance is relatively small, while with increased vertical compressive stress the effect of increasing material strength becomes significant and results in increased shear resistance. There is an interaction between the block and mortar strength such that the wall behavior may not be adequately predicted by considering the two strengths independently.

TABLE OF CONTENTS

	<u>Page</u>
ABSTRACT	iii
LIST OF TABLES	v
LIST OF FIGURES	vi
1. INTRODUCTION	1
2. TEST SPECIMENS	2
2.1 MATERIALS	2
2.2 WALL DETAILS	2
2.3 TEST SETUP AND INSTRUMENTATION	2
2.4 TEST PROCEDURE	6
3. DISCUSSION OF RESULTS	15
3.1 GENERAL BEHAVIOR	15
3.2 MAXIMUM IN-PLANE RESISTANCE	15
3.3 SHEAR STRESS-DISPLACEMENT RELATIONSHIP	23
3.4 DIAGONAL STRAIN	23
4. SUMMARY AND CONCLUSIONS	36
4.1 SUMMARY	36
4.2 CONCLUSIONS	36
5. ACKNOWLEDGEMENTS	37
6. REFERENCES	38
APPENDICES	A-1
A. OVERVIEW OF MASONRY RESEARCH PROGRAM	A-1
B. MATERIALS	B-1
B.1 CONCRETE MASONRY UNITS	B-1
B.2 MORTAR	B-1
C. WALL PANEL DESCRIPTION	C-1
C.1 WALL PANEL FABRICATION	C-1
C.2 PRISM FABRICATION	C-1
D. WALL PANEL TESTS: SETUP AND PLACEMENT	D-1
D.1 TEST SETUP	D-1
D.2 WALL PANEL PLACEMENT	D-1
E. WALL PANEL LOAD-DISPLACEMENT CURVES	E-1

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TABLE OF CONTENTS

	<u>Page</u>
ABSTRACT	iii
LIST OF TABLES	v
LIST OF FIGURES	vi
1. INTRODUCTION	1
2. TEST SPECIMENS	2
2.1 MATERIALS	2
2.2 WALL DETAILS	2
2.3 TEST SETUP AND INSTRUMENTATION	2
2.4 TEST PROCEDURE	6
3. DISCUSSION OF RESULTS	15
3.1 GENERAL BEHAVIOR	15
3.2 MAXIMUM IN-PLANE RESISTANCE	15
3.3 SHEAR STRESS-DISPLACEMENT RELATIONSHIP	23
3.4 DIAGONAL STRAIN	23
4. SUMMARY AND CONCLUSIONS	36
4.1 SUMMARY	36
4.2 CONCLUSIONS	36
5. ACKNOWLEDGEMENTS	37
6. REFERENCES	38
APPENDICES	A-1
A. OVERVIEW OF MASONRY RESEARCH PROGRAM	A-1
B. MATERIALS	B-1
B.1 CONCRETE MASONRY UNITS	B-1
B.2 MORTAR	B-1
C. WALL PANEL DESCRIPTION	C-1
C.1 WALL PANEL FABRICATION	C-1
C.2 PRISM FABRICATION	C-1
D. WALL PANEL TESTS: SETUP AND PLACEMENT	D-1
D.1 TEST SETUP	D-1
D.2 WALL PANEL PLACEMENT	D-1
E. WALL PANEL LOAD-DISPLACEMENT CURVES	E-1

LIST OF TABLES

	<u>Page</u>
Table 2.1 Wall Panel Details	4
Table 2.2 Wall Panel Horizontal LVDT Location Dimensions	8
Table 3.1 Wall Panel Maximum In-plane Resistance	20
Table B.1 Dimensions and Properties of Concrete Masonry Units	B-3
Table B.2 Masonry Sand Sieve Analysis	B-5

LIST OF FIGURES

	<u>Page</u>
Figure 2.1	Typical wall panel 3
Figure 2.2	Test setup 5
Figure 2.3	Wall panel LVDT layout 7
Figure 2.4	Diagonal LVDT 9
Figure 2.5	Leaf spring transducer 10
Figure 2.6	Wall panel instrumentation (48 in.) 11
Figure 2.7	Wall panel instrumentation (64 in.) 12
Figure 2.8	Wall panel instrumentation (96 in.) 13
Figure 2.9	Simplified description of test method 14
Figure 3.1	Typical crack pattern (64HL240) 16
Figure 3.2	Typical crack pattern (64LH340) 16
Figure 3.3	Flexural cracking (64LH105) 17
Figure 3.4	Crack pattern with high vertical stress (64HL320) 17
Figure 3.5	Crack pattern in low strength wall (64LL250) 18
Figure 3.6	Maximum shear stress versus vertical stress for 48 in. and 96 in. walls 21
Figure 3.7	Maximum shear stress versus vertical stress for 64 in. walls 21
Figure 3.8	Cracking in 96 in. wall panels 22
Figure 3.9	Regression analysis curves 22
Figure 3.10	Shear stress-displacement curves (64HH & 64LH) 24
Figure 3.11	Shear stress-displacement curves (64HL & 64LL) 25
Figure 3.12	Shear stress-displacement curves (64HH & 64HL) 26
Figure 3.13	Shear stress-displacement curves (64LH & 64LL) 27
Figure 3.14	Shear stress-displacement curves (48 in.) 28
Figure 3.15	Shear stress-displacement curves (96 in.) 29
Figure 3.16	Combined shear stress-diagonal strain curves 31
Figure 3.17	Shear stress-diagonal strain curves (64HH & 64LH) 32
Figure 3.18	Shear stress-diagonal strain curves (64HL & 64LL) 33
Figure 3.19	Shear stress-diagonal strain curves (64HH & 64HL) 34
Figure 3.20	Shear stress-diagonal strain curves (64LH & 64LL) 35
Figure B.1	Concrete Masonry Units B-4
Figure D.1	Wall panel transport harness D-2
Figure D.2	Placement of a wall panel D-2
Figure E.1	Crack pattern and load-displacement curve for specimen 64HL160 E-2
Figure E.2	Crack pattern and load-displacement curve for specimen 64HL240 E-3
Figure E.3	Crack pattern and load-displacement curve for specimen 64HL320 E-4
Figure E.4	Crack pattern and load-displacement curve for specimen 64HL400 E-5
Figure E.5	Crack pattern and load-displacement curve for specimen 64LL170 E-6
Figure E.6	Crack pattern and load-displacement curve for specimen 64LL250 E-7

LIST OF FIGURES (CONT.)

	<u>Page</u>
Figure E.7 Crack pattern and load-displacement curve for Specimen 64LL340	E- 8
Figure E.8 Crack pattern and load-displacement curve for specimen 64LL420	E- 9
Figure E.9 Crack pattern and load-displacement curve for specimen 64LH105	E-10
Figure E.10 Crack pattern and load-displacement curve for specimen 64LH170	E-11
Figure E.11 Crack pattern and load-displacement curve for specimen 64LH250	E-12
Figure E.12 Crack pattern and load-displacement curve for specimen 64LH340	E-13
Figure E.13 Crack pattern and load-displacement curve for specimen 64LH420	E-14
Figure E.14 Crack pattern and load-displacement curve for specimen 48LH170	E-15
Figure E.15 Crack pattern and load-displacement curve for specimen 48LH450	E-16
Figure E.16 Crack pattern and load-displacement curve for specimen 96LH220	E-17
Figure E.17 Crack pattern and load-displacement curve for specimen 96LH320	E-18

1. INTRODUCTION

This report describes an experimental test series investigating the shear behavior of unreinforced, hollow concrete block masonry walls. The test series reported herein is part of a larger experimental research program which has as its main objective the determination of the in-plane shear (lateral load) resistance of shear dominated masonry walls. The primary parameters discussed in this report are block and mortar strength. The vertical (axial) compressive stress and wall aspect ratio are also parameters in this test series, but only secondarily, in order to observe the interaction of parameters.

Data from seventeen wall tests are presented in this report. The walls have nominal heights and thicknesses of 64 in. and 8 in., respectively. Thirteen of the seventeen walls are 64 in. long, two of the walls are 48 in. and the remaining two walls are 96 in. long. The vertical compressive stress varies in the test series from a low of about 100 psi to a high of approximately 400 psi. These stresses are based on the net cross-sectional area of the walls. The concrete block has a gross area unit strength of either 1300 psi or 1800 psi (approximate) and the mortar is proportioned either as a Type S or a Type N.

This report is the third in a series of reports documenting the overall research program. Since the report is but one in a series, certain information common to all of the reports has been placed in appendices. The intent of this organization is to reduce redundancy in the main body of the report without omitting information necessary to the report's understanding if read independently of the other reports in the series. The purpose of this report is to present in a timely manner a general description of the tests and a limited presentation of key data. Detailed data analysis is left to summary reports which are issued periodically and combine sufficient test information to fully address a particular topic.

A brief background description of the overall research program is in appendix A. The materials and wall panel descriptions are discussed in chapter 2 along with instrumentation and general test procedure. Further descriptions of materials, fabrication techniques, and test setup may be found in appendices B, C, and D. A brief, general discussion of the more obvious test results is presented in chapter 3. The individual wall panel load-displacement curves and crack pattern maps are in appendix E. The summary and conclusions are presented in chapter 4.

2. TEST SPECIMENS

This chapter presents a brief description of the materials used in constructing the walls, the details of each wall (e.g., length, block strength), instrumentation, and general test procedures. More complete descriptions may be found in appendices B, C, and D.

2.1 MATERIALS

All materials used in the wall panels and companion prisms were commercially available and were representative of those commonly used in building construction. The concrete block was hollow, two-core block and was from two lots. One lot contained block having a gross area unit compressive strength of 1813 psi while the other lot had block whose gross area unit compressive strength was 1304 psi. For the purposes of this report the block having the strength of 1813 psi was identified as "high" strength, while the other block was termed "low" strength. These terms were relative indices and not a classification of their absolute strengths. The mortar used in constructing the walls was proportioned as either a Type S and termed "high" strength or as a Type N and designated "low" strength.

2.2 WALL DETAILS

A typical wall panel is illustrated in fig. 2.1. The details of each wall are listed in table 2.1. The wall panel identifier is a two part label with the two parts separated by a hyphen. That part of the identifier preceding the hyphen is the descriptive label and has the form mABn. In place of m there is a number representing the nominal length of the wall panel expressed in inches. The n term is the approximate applied vertical compressive stress on the wall panel. The stress is based on the net cross-sectional area of the wall and is in units of psi. The positions represented by A and B designate the strength of block and mortar, respectively. The letter H signifies "high" while the letter L denotes "low". An example of the descriptive label is 64HL240 which is read to mean a 64 in. long wall constructed with "high" strength block and "low" strength mortar which is tested with an applied vertical compressive stress of about 240 psi.

2.3 TEST SETUP AND INSTRUMENTATION

The walls are tested in the NBS Tri-directional Test Facility (fig. 2.2). The facility is a structural test rig which can apply forces and/or displacements in any or all of the six degrees of freedom at the top of the wall. The bottom of the wall is held stationary by a restraining crosshead.

The instrumentation used to monitor the behavior of a wall panel during a test can be divided into two main categories. The first category includes the load and displacement transducers mounted on the hydraulic actuators which are part of the test facility. The information from these transducers is used both for the control system of the test facility and for measuring specimen response. The global forces and displacements imposed on a test specimen are computed from these data. The second category of instrumentation includes the

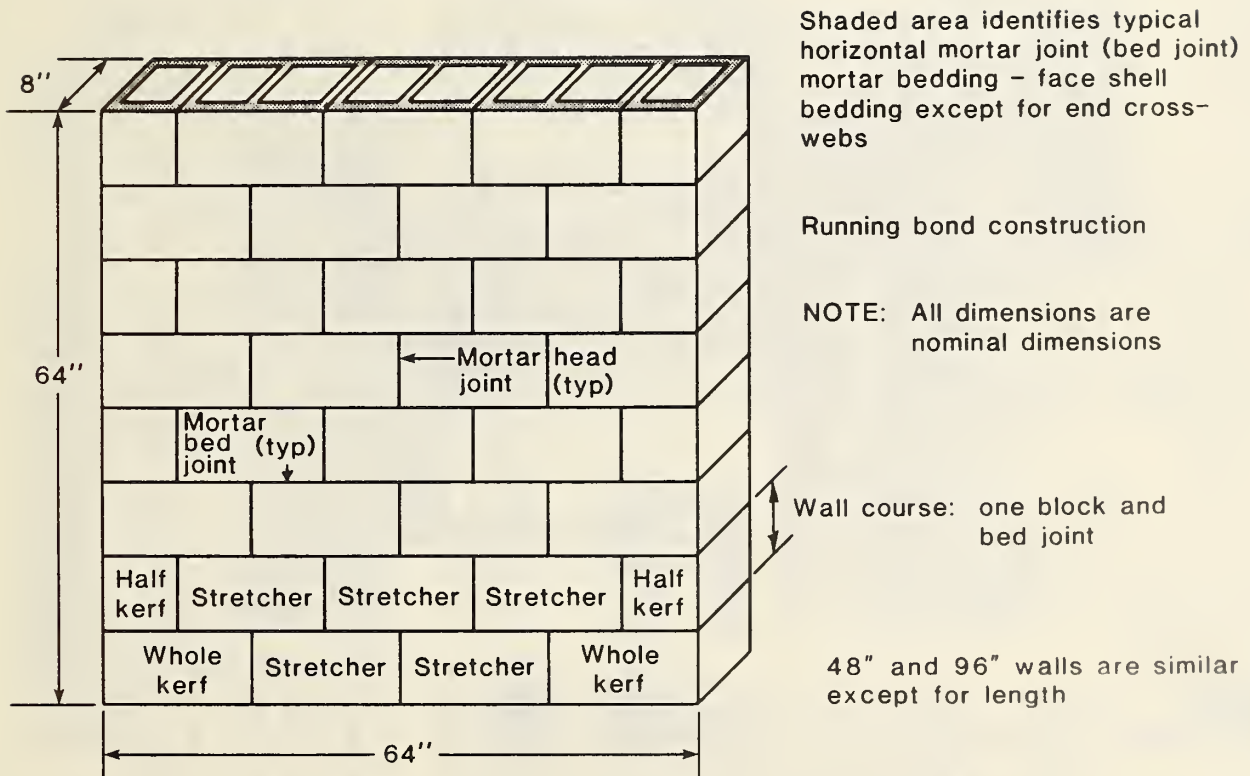


Figure 2.1 Typical wall panel

Table 2.1 Wall Panel Details

Wall Panel Identifier	Wall Age at Test (Days)	Mortar Cube Ultimate Compressive Strength*		Prism Bedding	Prism Ultimate Compressive Strength**	
		28 Day - (PSI) -	[DAYS]		PSI	[DAYS]
64HL160-5L01	121	1826	- 1627	Face Shell	2049	[48]
64HL240-5L02	120	1809	- 1692	- - - -	---	
64HL320-5L03	119	1761	- 1810	- - - -	---	
64HL400-5L04	118	1490	- 1684	Face Shell	1923	[50]
64LL170-6L07	103	1987	- 2253	Face Shell	1522	[58]
64LL250-6L06	99	1841	- 1892	Fully bedded	1955	[57]
64LL340-6L08	98	1591	- 2023	Fully bedded	1983	[61]
64LL420-6L09	96	1505	- 1896	Face Shell	1443	[62]
64LH105-6L01	113	2646	- 3074	Face Shell	1630	[51]
64LH170-6L02	112	2657	- 2906	Fully bedded	2033	[55]
64LH250-6L03	111	2772	- 2667	Face Shell	1522	[54]
64LH340-6L04	106	3127	- 3729	Fully bedded	2133	[56]
64LH420-6L05	104	3110	- 3754	- - - -	---	
48LH170-6L10	89	2985	- 3125	Face Shell	1447	[57]
48LH450-6L11	86	2892	- 3171	Fully bedded	2094	[62]
96LH220-6L12	83	2700	- 2914	Face Shell	1537	[63]
96LH320-6L13	79	2810	- 2775	Fully bedded	2025	[64]

* The stress is based on an area of 4 sq. in. The first value is the average of three cube tests at 28 days and the second value is the average of three cube tests at the number of days shown in square brackets. Mortar cubes were removed from the molds after 24 hours and air cured in the laboratory environment until testing.

** Each stress value is the average of at least three prism tests performed at the age shown in square brackets. The area used in calculation is the appropriate solid area listed in table B.1.

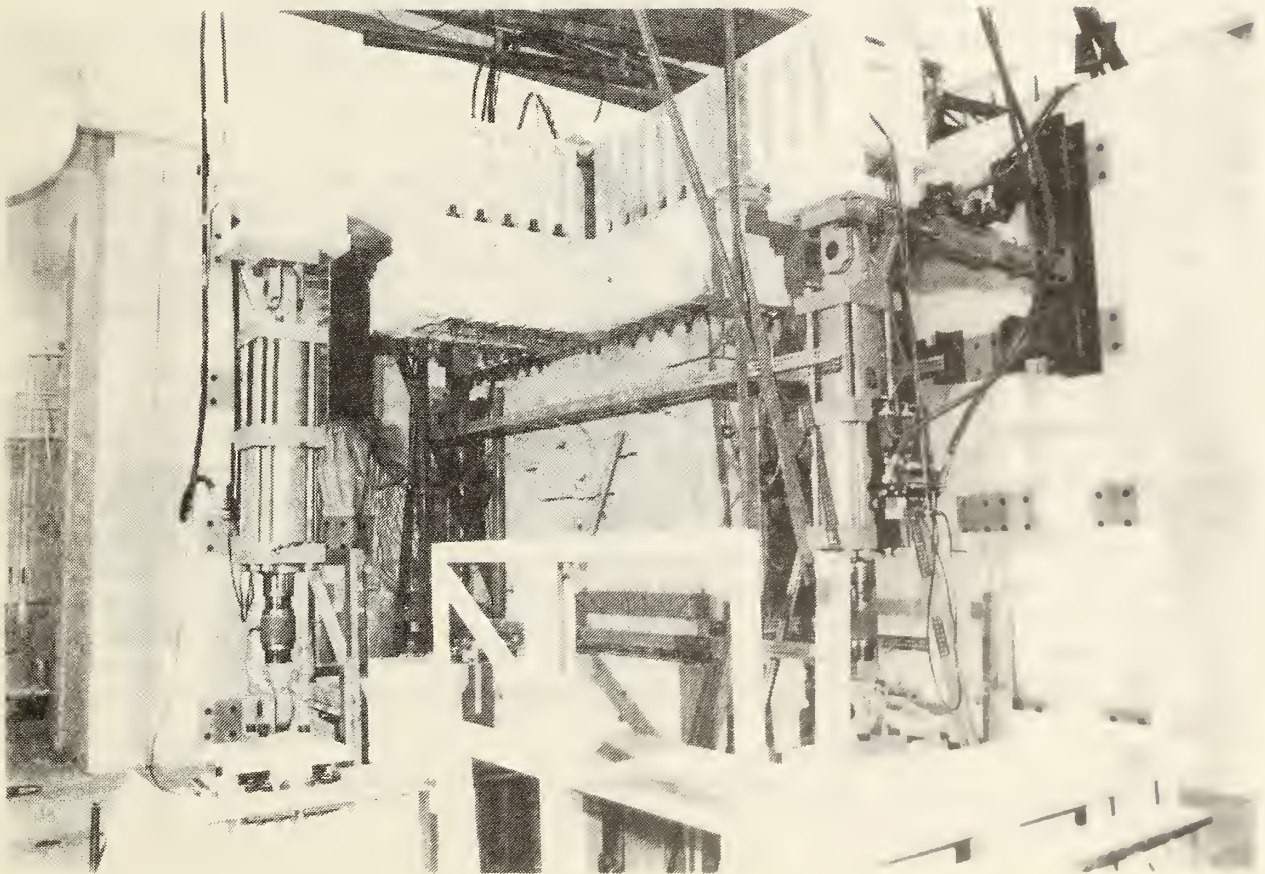


Figure 2.2 Test setup

transducers physically attached to the wall specimens. These include horizontally mounted linear variable differential transformers (LVDTs) which are mounted to measure the in-plane displacement of the wall along the wall height. The displacements are measured relative to a fixed reference. The positions of the LVDTs are shown in fig. 2.3 and listed in table 2.2. The other instrumentation attached to the test specimen measure displacements of one part of the wall relative to another part of the wall. These data are then used to compute wall strains by dividing the measured displacement by the gage length of the transducers. Two general types of transducers are used, the first being LVDTs mounted between swiveling posts attached to the wall surface (fig. 2.4). These devices are used only for measuring displacement along the wall diagonals, one on each side of the wall. The other type of transducer is a leaf spring transducer also mounted on posts attached to the wall surface (fig. 2.5). The general locations of the transducers mounted on the wall panels are shown in figures 2.6, 2.7, and 2.8 for the 48 in., 64 in., and 96 in. long walls, respectively.

All of the analog instrumentation is connected to a high speed analog-to-digital converter which is part of a general computer-based data acquisition and load control system.

2.4 TEST PROCEDURE

The first step in a typical wall test was to check the data channels for unusual variations in output and obtain a measure of the ambient voltage oscillation. With the data channels in proper working order a set of data was acquired which served as the "zero" condition of the test. The hydraulic actuators of the test facility were then pressurized and another set of data readings were obtained. Compressive vertical (axial) load was applied to the wall until the desired value was achieved. After stabilizing the vertical load, in-plane lateral displacement was applied at the top of the wall panel. The lateral displacement was applied with the upper crosshead of the facility maintaining a "zero" rotation condition (fig. 2.9). The vertical load was automatically maintained at the preset level by the facility's control system. The initial direction of lateral displacement was always to the west, but the displacement pattern varied slightly between tests. The lateral displacement was generally increased in the initial direction until a diagonal crack was fully formed. Afterwards, the displacement was either reversed or increased until the wall could not support the imposed vertical load. Data were acquired at regular intervals during the test at lateral displacement intervals of about 0.005 in.

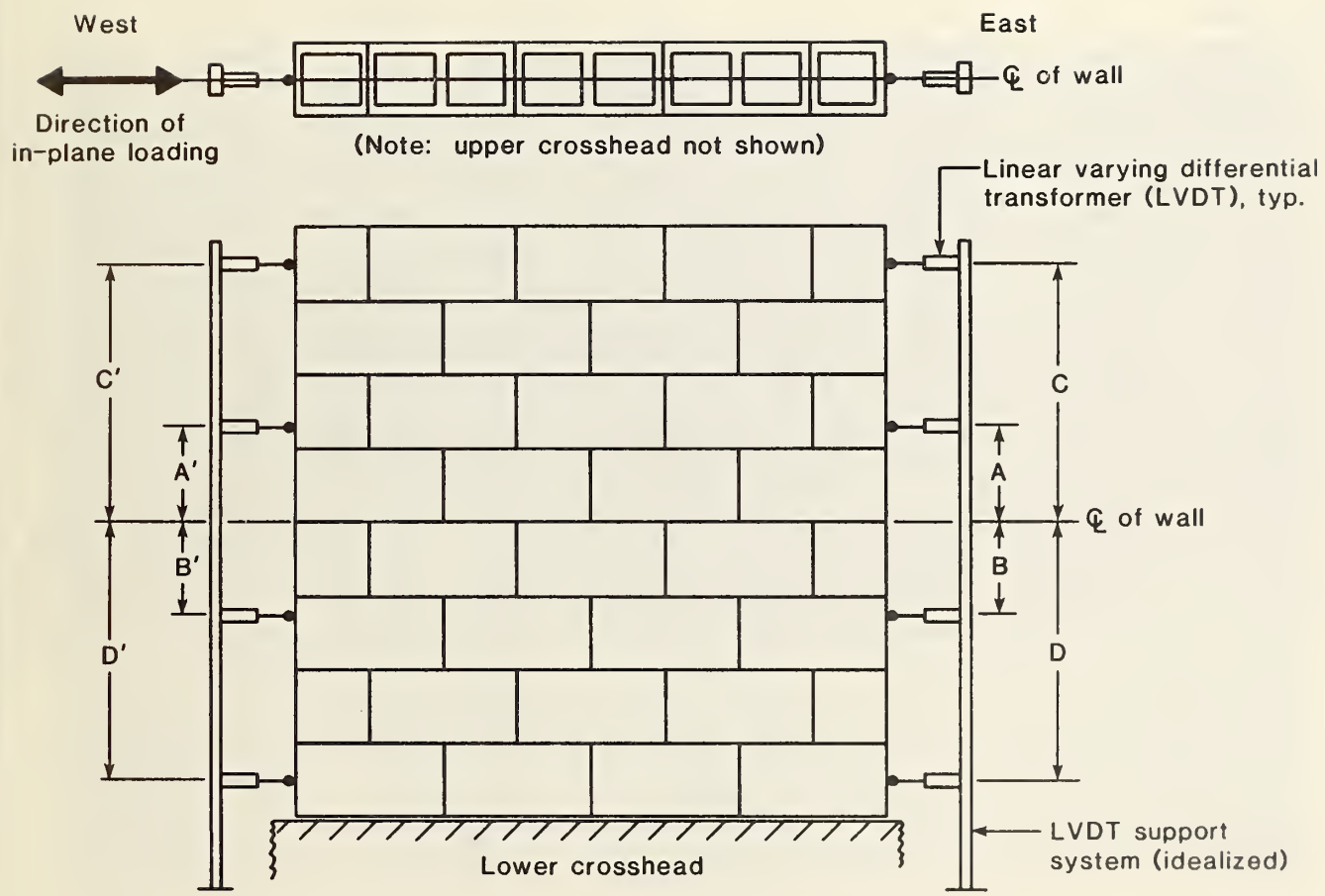


Figure 2.3 Wall panel LVDT layout

Table 2.2 Wall Panel Horizontal LVDT Location Dimensions

(Refer to fig. 2.3 for identification of dimension locations A, B, C, D, A', B', C', and D')

Wall Panel Identifier	A (in.)	B (in.)	C (in.)	D (in.)	A' (in.)	B' (in.)	C' (in.)	D' (in.)
64HL160	10.25	9.50	27.13	29.25	9.88	10.88	27.25	28.25
64HL240	9.88	9.88	26.75	29.50	10.0	10.38	27.50	27.75
64HL320	10.38	9.50	27.13	29.25	7.38	10.38	27.63	27.38
64HL400	10.25	9.38	27.0	29.13	7.38	10.38	27.75	27.38
64LL170	10.25	9.13	27.50	28.50	9.50	10.0	27.88	27.13
64LL250	10.38	9.13	27.38	28.63	9.25	10.0	27.63	27.38
64LL340	10.13	9.25	27.38	28.75	9.25	10.25	27.50	27.38
64LL420	10.13	9.25	27.38	28.75	9.25	10.25	27.50	27.38
64LH105	10.0	9.75	26.75	29.38	7.13	10.75	27.50	27.75
64LH170	9.75	9.75	26.88	29.25	8.75	10.75	27.0	27.88
64LH250	9.75	9.63	26.75	29.50	8.63	10.63	27.13	28.0
64LH340	10.0	9.63	26.75	29.50	8.63	10.75	26.88	28.0
64LH420	10.0	9.63	26.75	29.50	8.63	10.75	26.88	28.0
48LH170	10.13	9.25	27.88	28.50	9.38	10.0	27.88	27.38
48LH450	10.63	9.13	28.25	28.13	9.38	10.25	27.75	27.38
96LH220	10.50	9.0	28.50	27.75	9.38	10.13	27.75	27.25
96LH320	10.50	9.0	28.50	27.75	9.38	10.13	27.75	27.25

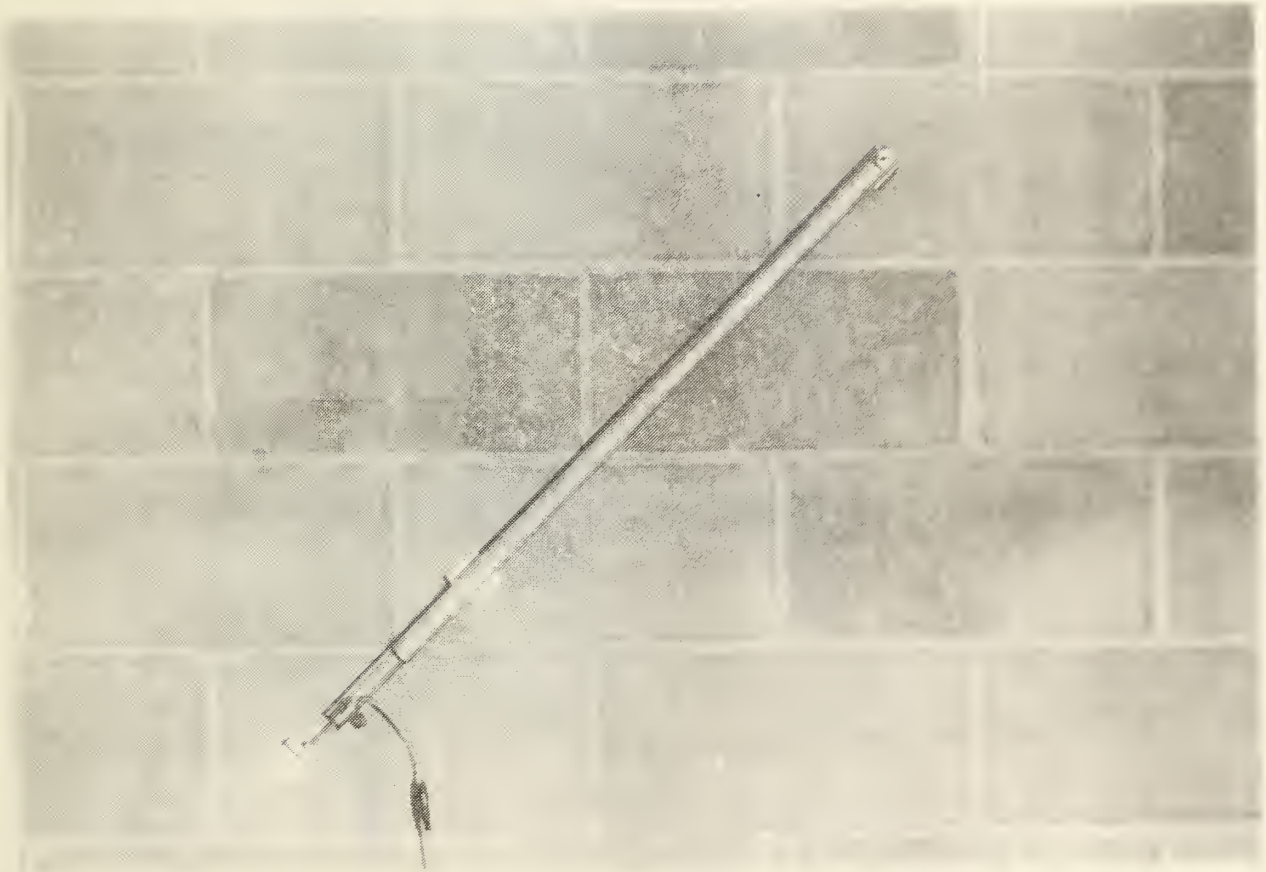


Figure 2.4 Diagonal LVDT

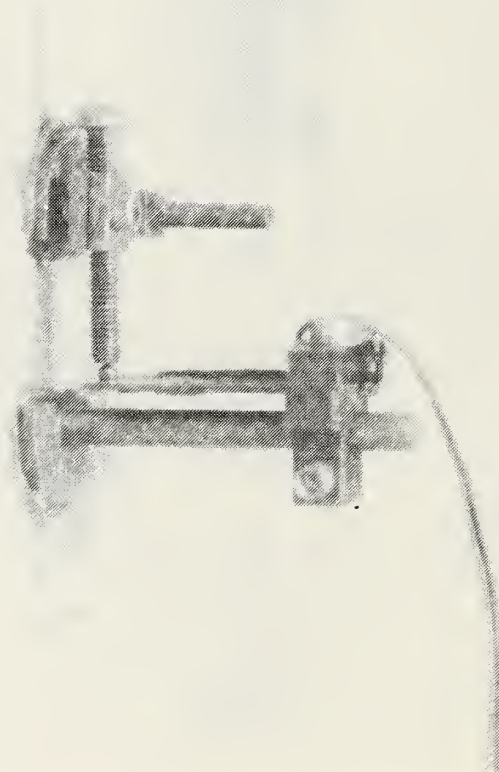
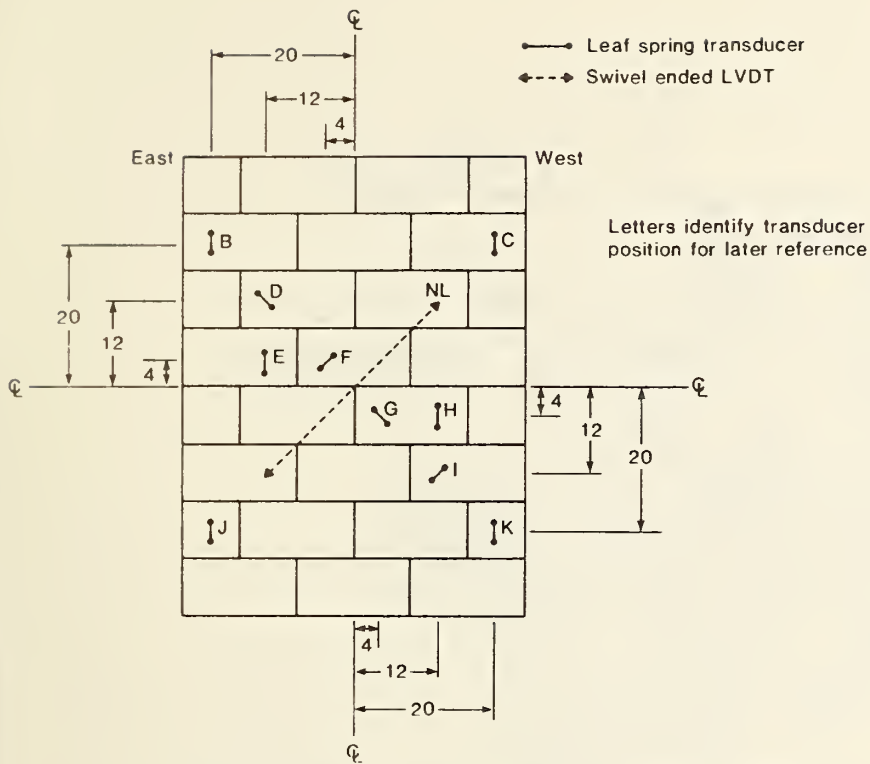
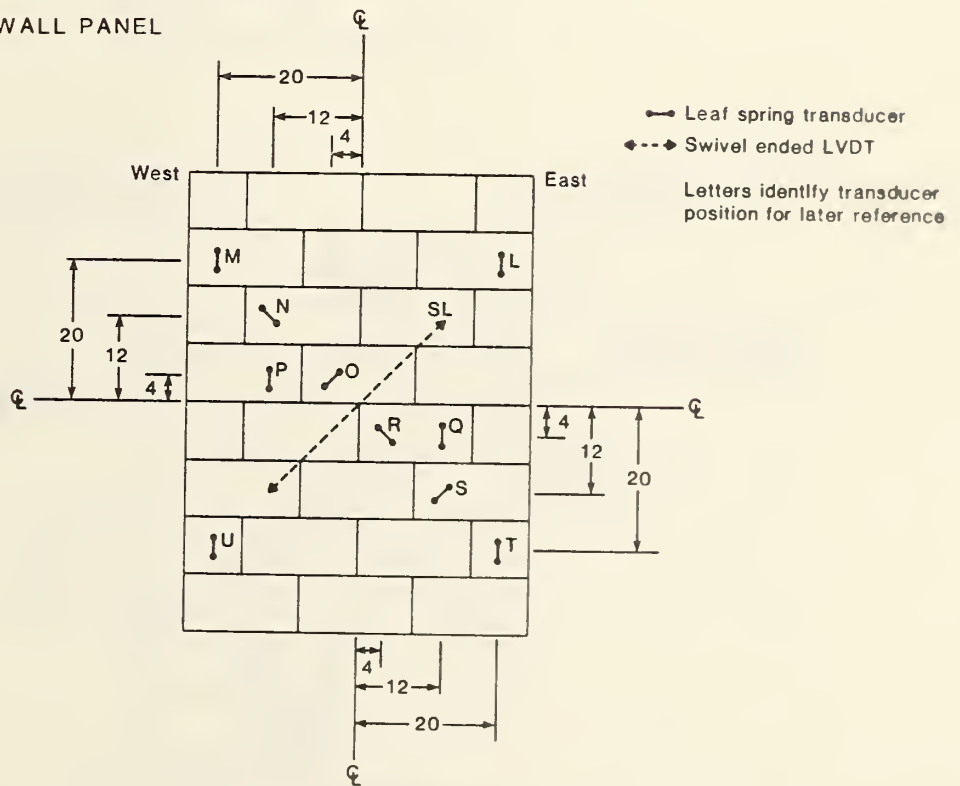


Figure 2.5 Leaf spring transducer



NORTH SIDE OF 48" WALL PANEL



SOUTH SIDE OF 48" WALL PANEL

Figure 2.6 Wall panel instrumentation (48 in.)

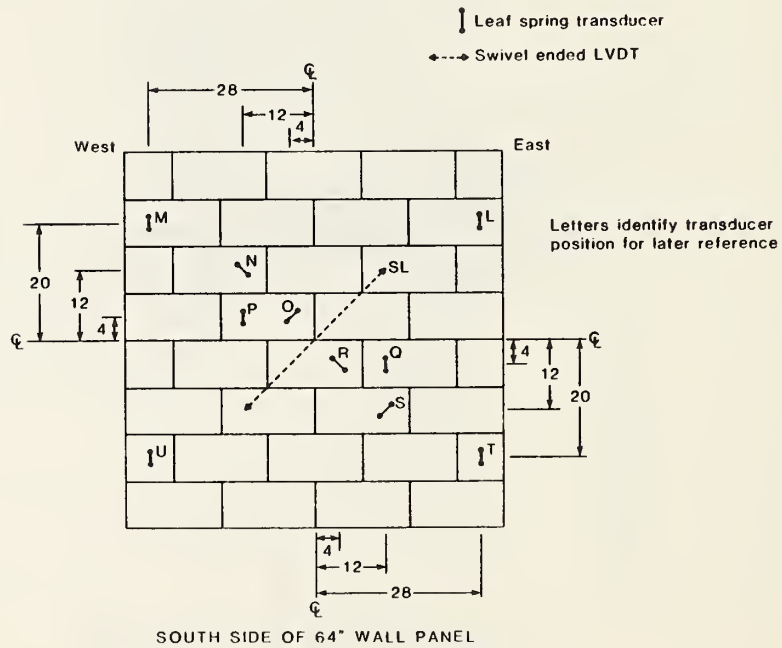
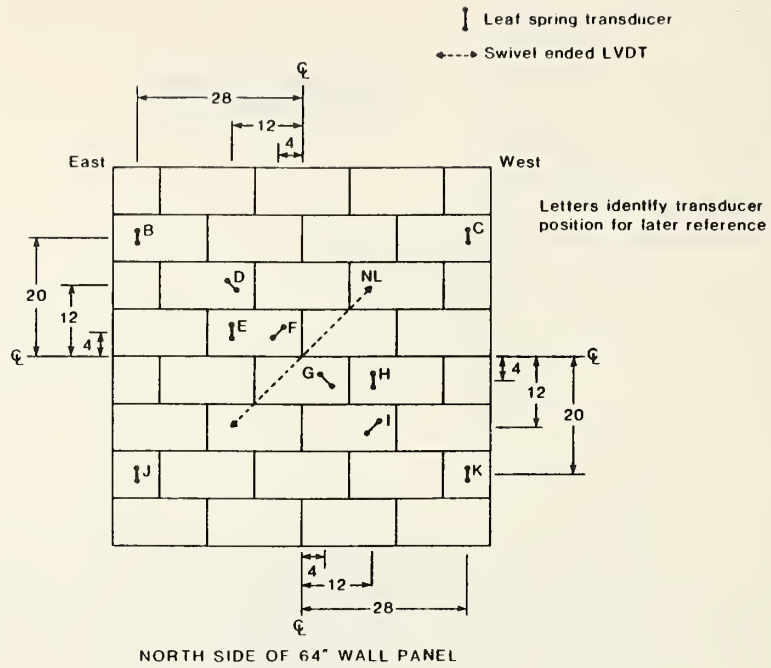


Figure 2.7 Wall panel instrumentation (64 in.)

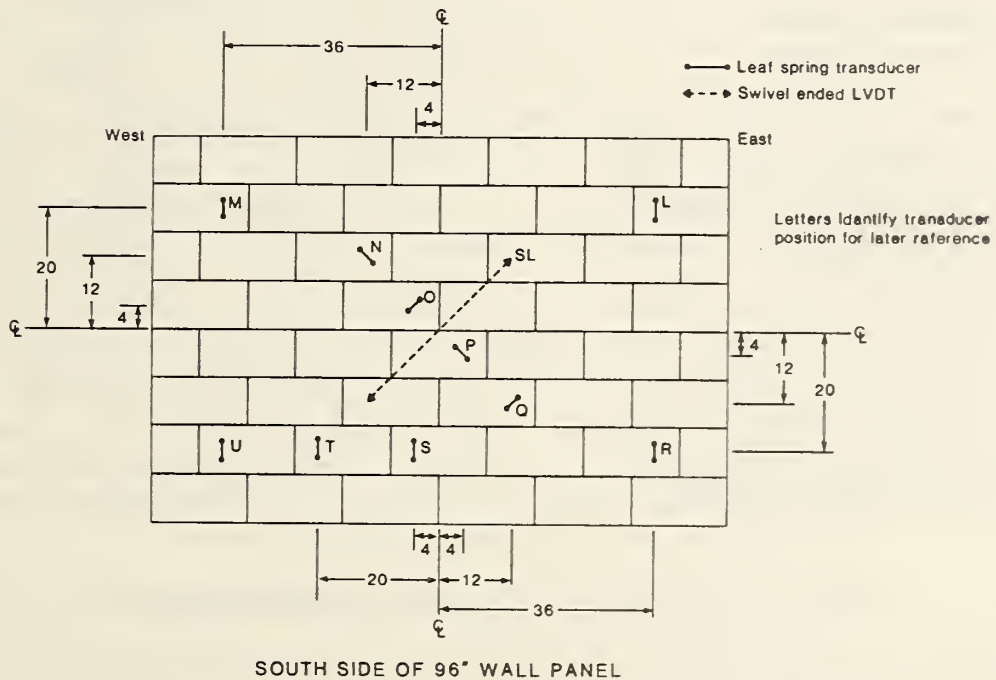
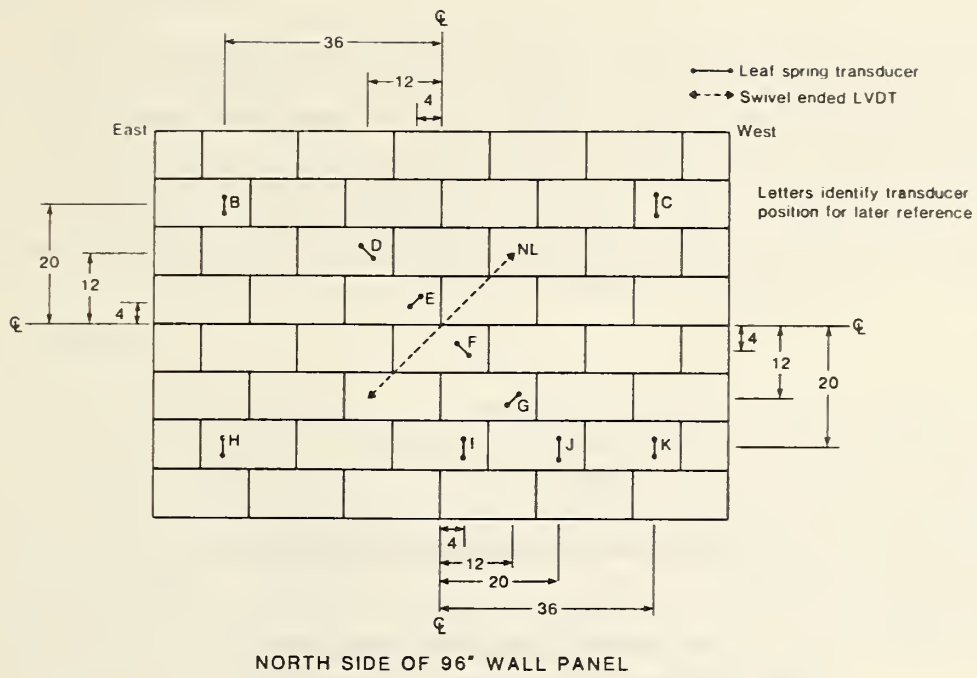
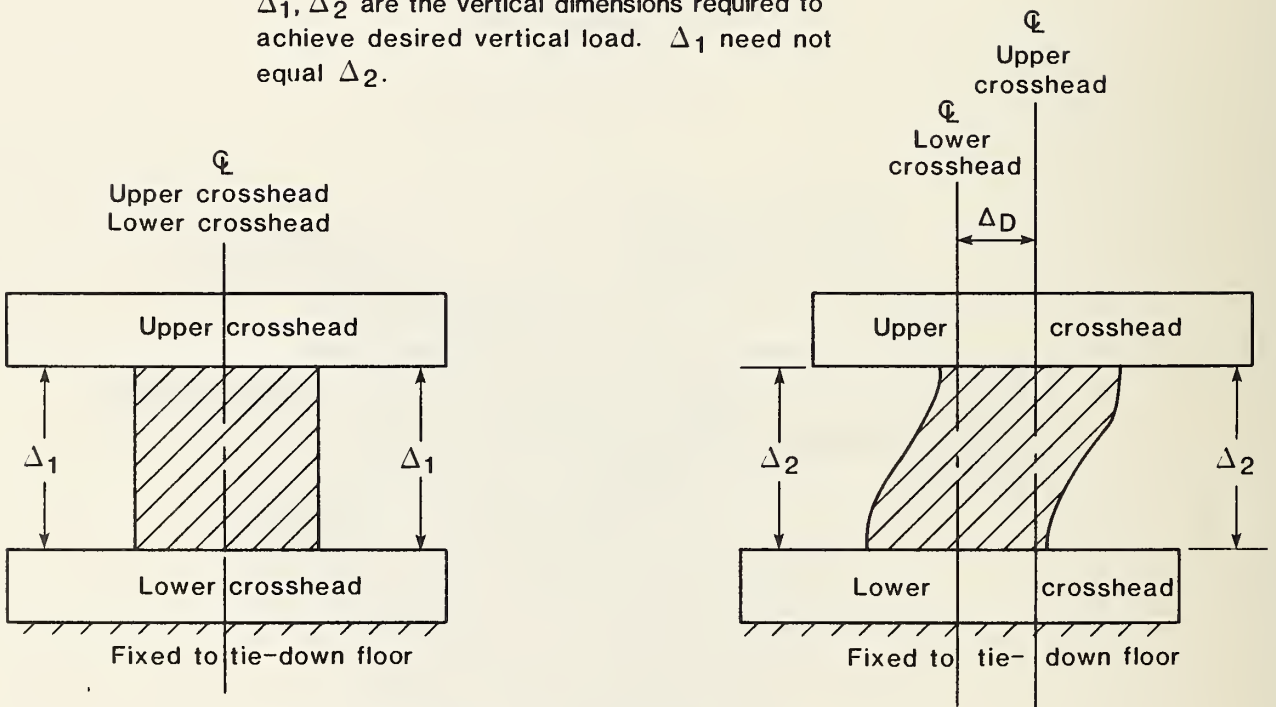


Figure 2.8 Wall panel instrumentation (96 in.)

Δ_D is the imposed in-plane lateral displacement

Δ_1, Δ_2 are the vertical dimensions required to achieve desired vertical load. Δ_1 need not equal Δ_2 .



SIMPLIFIED DESCRIPTION OF IMPOSED DISPLACEMENT

Figure 2.9 Simplified description of test method

3. DISCUSSION OF RESULTS

In addition to the seventeen wall panel tests reported herein, data from wall panel tests reported previously [1, 2] are used in the discussion which follows. The combination of wall tests include variations in applied vertical compressive stress and wall panel aspect ratio. The focus of the discussion is on the influence of block and mortar strength, but effects of the other parameters will be discussed when it appears that there is an interaction between the parameters. The presentation is not intended to be a detailed analysis of the data as that will appear in a subsequent summary report.

3.1 GENERAL BEHAVIOR

The typical mode of wall distress was a diagonal tension form of shear distress as evidenced by the pronounced cracking along the diagonals of the wall panels (figures 3.1 and 3.2). The exception to this mode of distress came only when the level of vertical compressive stress was low, resulting in a flexurally dominated initial mode of distress as shown in fig. 3.3 where the horizontal flexure crack developed. However, even for the walls exhibiting flexural distress, it was possible to produce a local diagonal tension crack and, if the vertical compressive stress was increased sufficiently, a general diagonal tension mode of distress.

Given that the typical indication of distress was diagonal tension cracking, there were observed differences in the form of the crack paths. It was previously observed [1, 2] that both the level of vertical compressive stress and the aspect ratio influenced the orientation of the cracking. In general, the trajectory of the crack pattern was rather constant along a roughly 45 degree line, especially for the square wall panels. The path that the diagonal crack followed, however, was slightly more variable with most of the previously reported walls exhibiting block cracking rather than a simple stair-step crack along the mortar joints. The previous walls all had high strength block and mortar. The influence of block and mortar strength seemed to be consistent with the mortar strength having more effect. The degree of influence was a function of the level of vertical compressive stress and to a lesser degree aspect ratio. Walls with high strength block and low strength mortar tended to exhibit diagonal cracking of the stair-step fashion (fig. 3.1), but with high levels of vertical stress some block cracking occurred (fig. 3.4). Conversely, walls with low strength block and high strength mortar exhibited much more through block cracking (fig. 3.2). The combination of low strength block and mortar resulted in a definite combination of the two types of crack path (fig. 3.5), though it appeared that the cracking began along the mortar joints and then, shortly afterward, initiated in the block units.

3.2 MAXIMUM IN-PLANE RESISTANCE

The capacity of particular interest in these tests is the maximum in-plane lateral load resistance. This load is referred to as the shear force capacity and when the shear force is divided by a nominal value for the wall's net cross-sectional area the resulting computed stress is termed the shear stress. The advantage of using shear stress rather than force is in the normalization

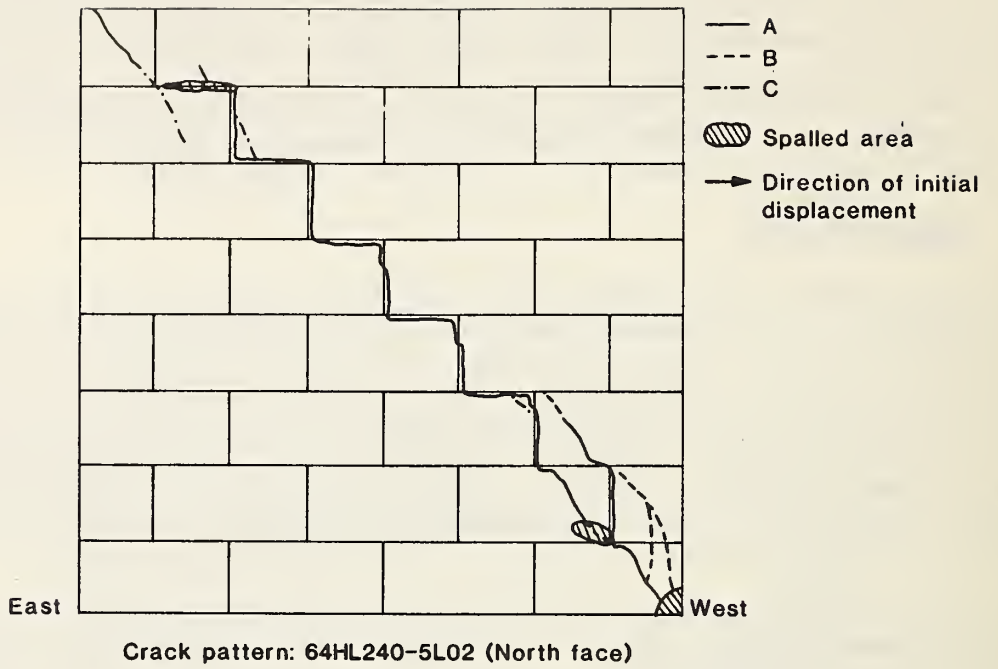


Figure 3.1 Typical crack pattern (64HL240)

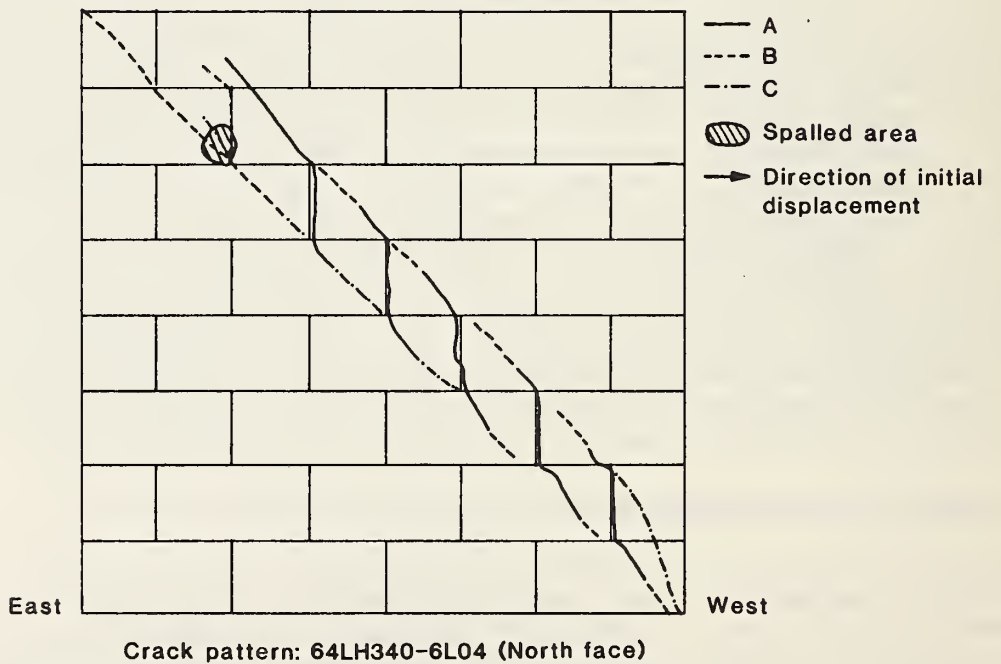
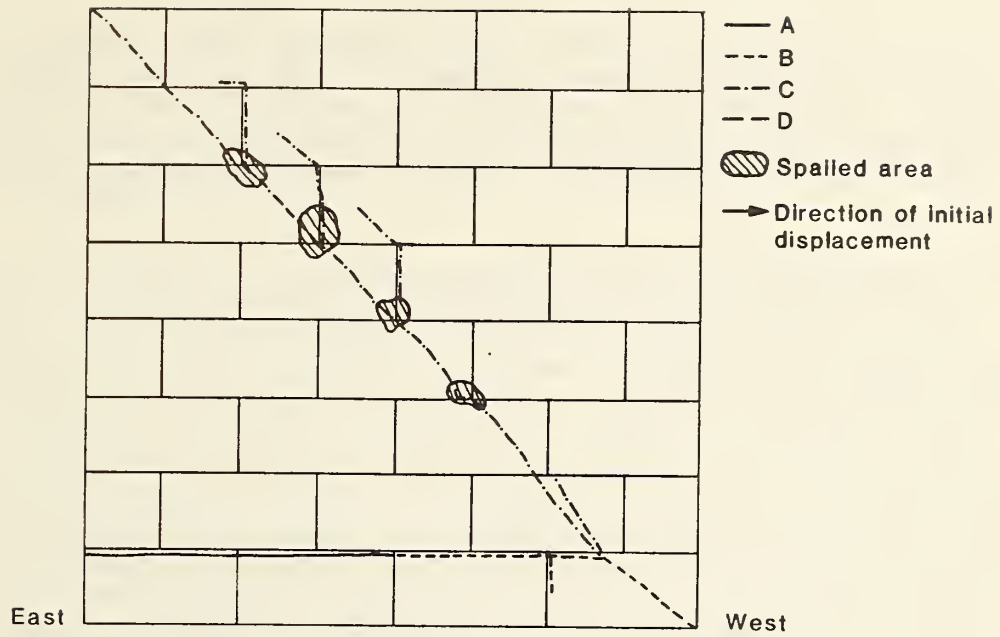
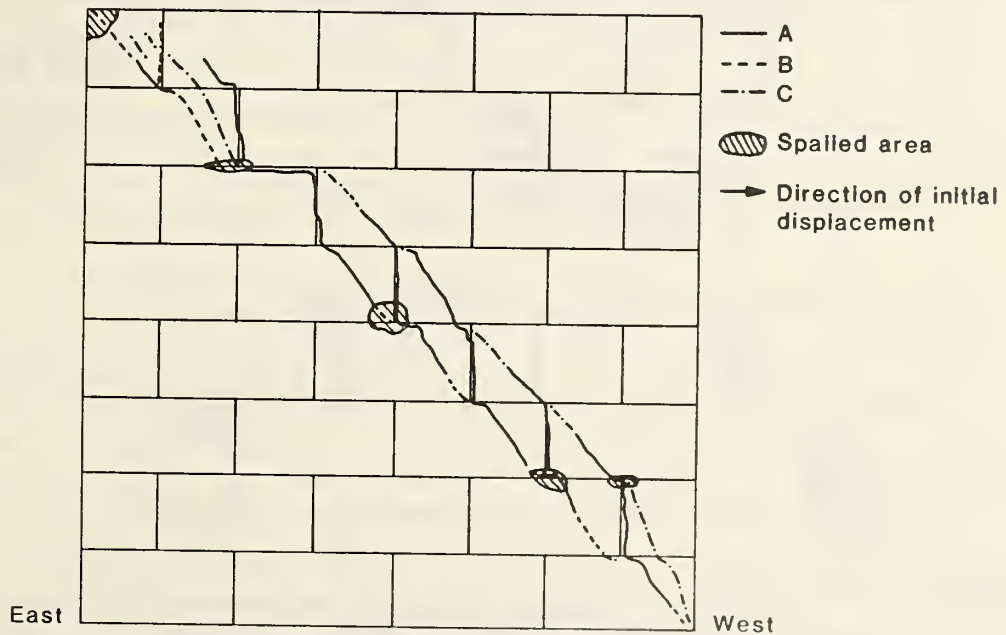


Figure 3.2 Typical crack pattern (64LH340)



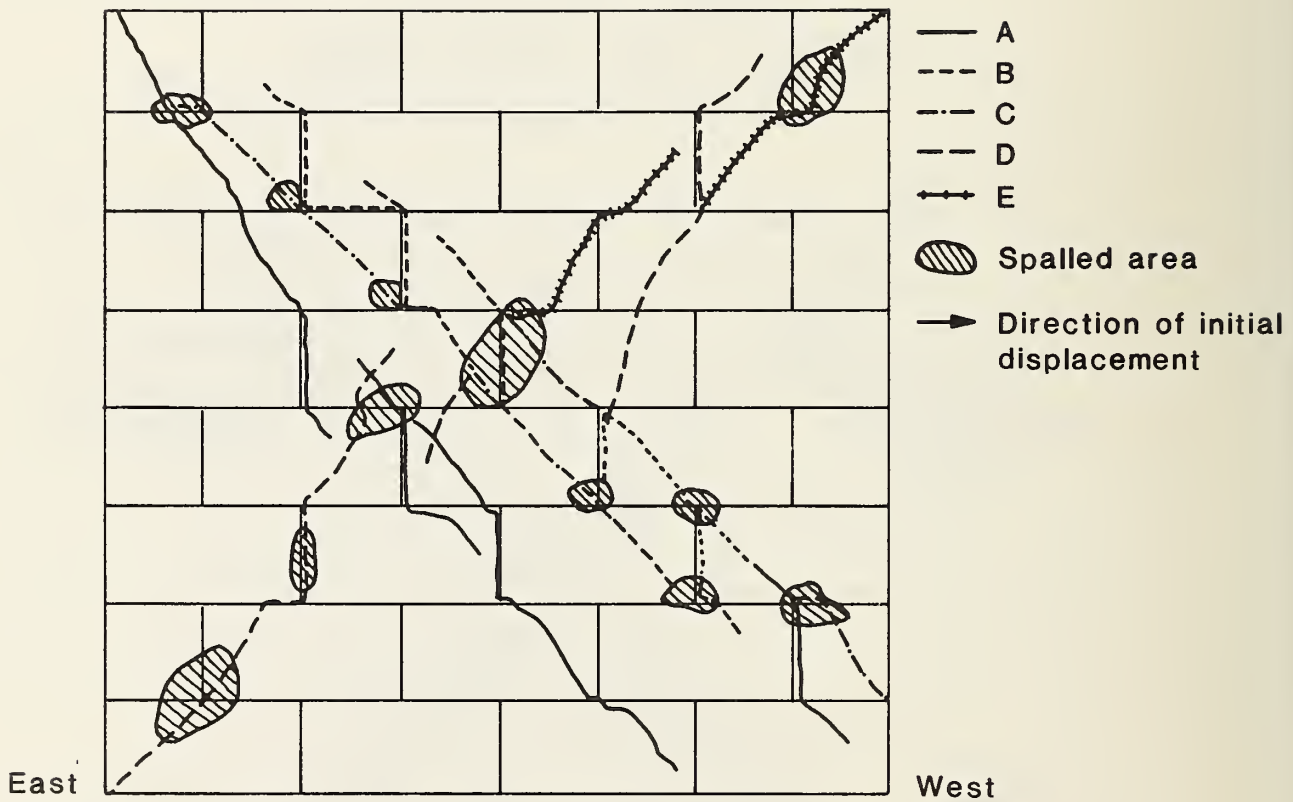
Crack pattern: 64LH105-6L01 (North face)

Figure 3.3 Flexural cracking (64LH105)



Crack pattern: 64HL320-5L03 (North face)

Figure 3.4 Crack pattern with high vertical stress (64HL320)



Crack pattern: 64LL250-6L06 (North face)

Figure 3.5 Crack pattern in low strength wall (64LL250)

of the capacity with respect to varying wall area as between a 96 in. wall and a 64 in. wall. This may be an incomplete normalization, but for the purposes of this report it is considered an acceptable normalization. The maximum shear resistance of each wall panel is listed in table 3.1 along with the applied vertical compression stress and wall displacement coincident with the achievement of maximum shear.

The maximum shear stresses are plotted versus the vertical compressive stresses in figures 3.6 and 3.7 along with data from the previously reported tests [1, 2]. Figure 3.6 contains the data for the 48 in. and 96 in. long wall panels. It appears that the block strength does not affect the maximum shear stress for the 96 in. walls and had only a minor impact for the 48 in. walls. In the case of the 48 in. walls the trend is, as might be expected, lower maximum shear resistance with the lower block strength. The trend, or more appropriately, the lack of a trend, for the 96 in. walls is most probably the result of the diagonal crack path. The diagonal cracking in the 96 in. walls was almost exclusively along the mortar joints (fig. 3.8) making it unlikely that the block strength would be a significant parameter for maximum shear stress. The mortar strength is not an intentional variable for the 48 in. and 96 in. long walls and no observations can be made on the effect of mortar strength.

The maximum shear stress versus vertical compressive stress comparisons for the 64 in. long wall panels are plotted in fig. 3.7. The comparison for block strength is made between the open and closed forms of each symbol and the comparison for mortar strength is made between similar forms, open or closed, of the different symbols. The most obvious general trend is the divergence of maximum shear stresses as the vertical stress increases. The group of data points at the 160 psi vertical compressive stress level reflect four different parameters and yet there is little difference between the points. By contrast, the spread in maximum shear stresses at the 400 psi vertical compressive stress level is quite large. It is clear that the vertical compressive stress has an effect on the apparent interaction between block and mortar strength and the maximum shear stress for the 64 in. long wall panels.

The trend of the data plotted in fig. 3.7 is more easily visualized by regression analyses (fig. 3.9). Four of the lines are linear while the remaining line, labelled 64LH, is quadratic because the regression analyses showed a significant improvement in fit for the quadratic compared to the linear representation. The line labels indicate the data used in determining each line. Label 64XX means that all of the data for the 64 in. long wall panels are used in the regression analysis for that line. A certain amount of caution should be exercised in evaluating the trends especially outside the limits of the data values. The bounds are indicated by vertical lines on the plot and describe the limits of stresses generally contained in the actual data. The tendency towards dispersion as the vertical stress increases is evident. Another clear trend is that as the vertical stress increases there is a marked decrease in maximum shear stress for the low strength block walls as compared to the high strength walls. A similar trend for mortar strength exists for the high strength block walls, but the trend is not as pronounced for the low strength block walls. This observation suggests that the total wall behavior may be

Table 3.1 Wall Panel Maximum In-plane Resistance

Wall Panel Identifier	Wall Net Cross-Sectional Area (in. ²)	Vertical Compressive Stress* (psi)	Maximum In-plane Load Resistance (kips)	Maximum In-plane Resistance* (psi)	Wall Displacement at Maximum Resistance (in.)
64HL160-5L01	246.0	163	30.0	122	0.090
64HL240-5L02	246.0	243	37.1	151	0.079
64HL320-5L03	246.0	316	42.1	171	0.086
64HL400-5L04	246.0	407	46.7	190	0.075
64LL170-6L07	241.6	162	27.7	115	0.067
64LL250-6L06	241.6	246	34.8	144	0.166
64LL340-6L08	241.6	332	38.2	158	0.069
64LL420-6L09	241.6	413	42.0	174	0.125
64LH105-6L01	241.6	103	24.3	101	0.120
64LH170-6L02	241.6	161	31.0	128	0.088
64LH250-6L03	241.6	248	36.7	152	0.079
64LH340-6L04	241.6	327	40.0	166	0.071
64LH420-6L05	241.6	418	42.7	177	0.074
48LH170-6L10	181.2	165	21.1	116	0.123
48LH450-6L11	181.2	430	29.3	162	0.073
96LH220-6L12	362.4	220	59.2	163	0.122
96LH320-6L13	362.4	315	73.7	203	0.154

* Values of stress are based on the net cross-sectional area of the walls.

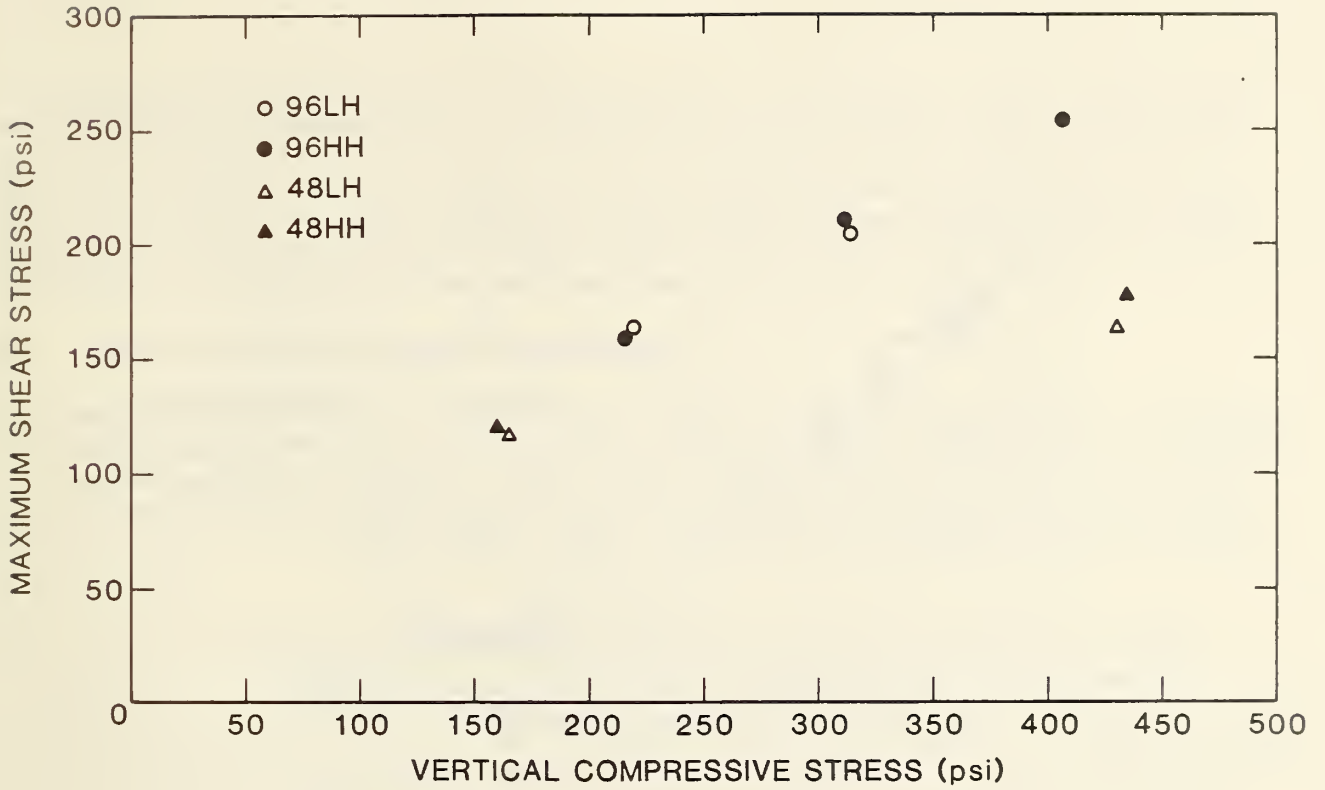


Figure 3.6 Maximum shear stress versus vertical stress for 48 in. and 96 in. walls

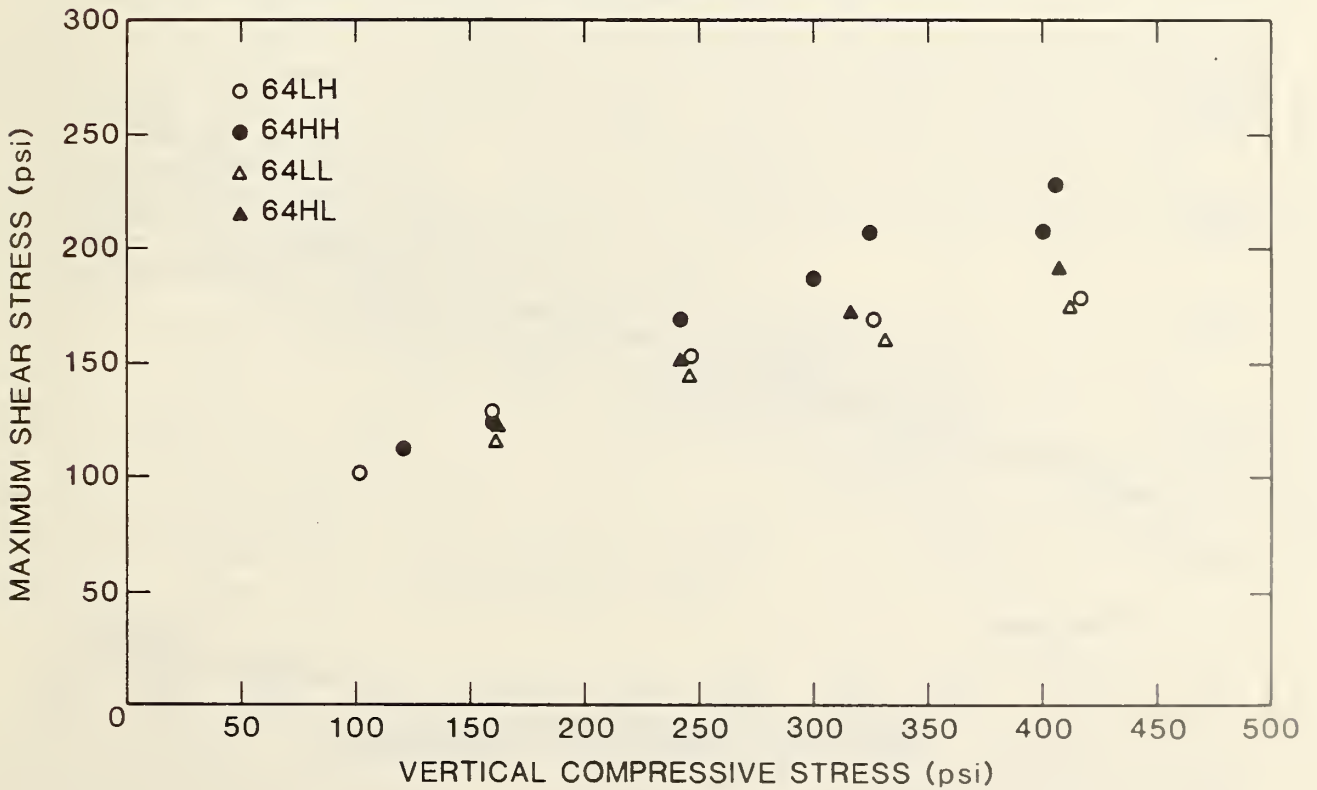


Figure 3.7 Maximum shear stress versus vertical stress for 64 in. walls

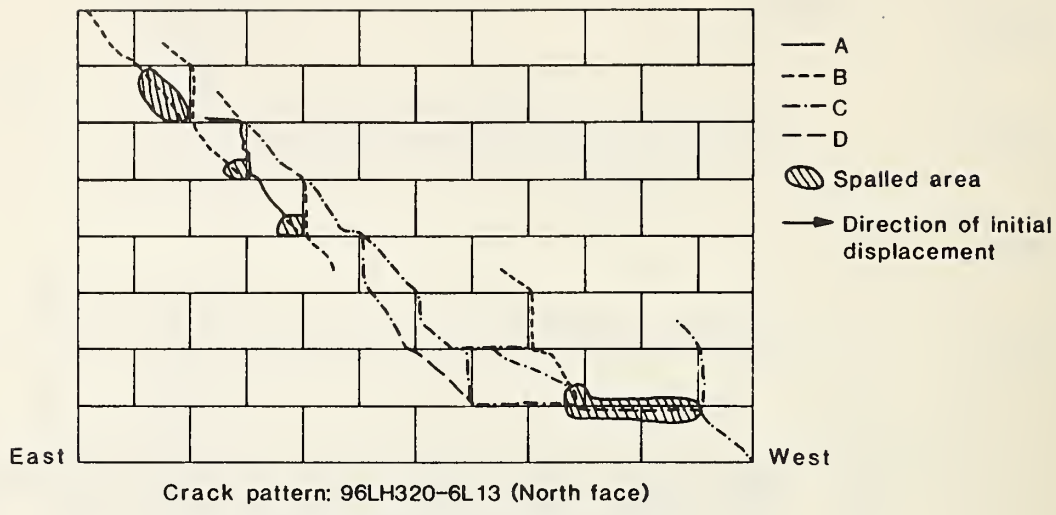


Figure 3.8 Cracking in 96 in. wall panels

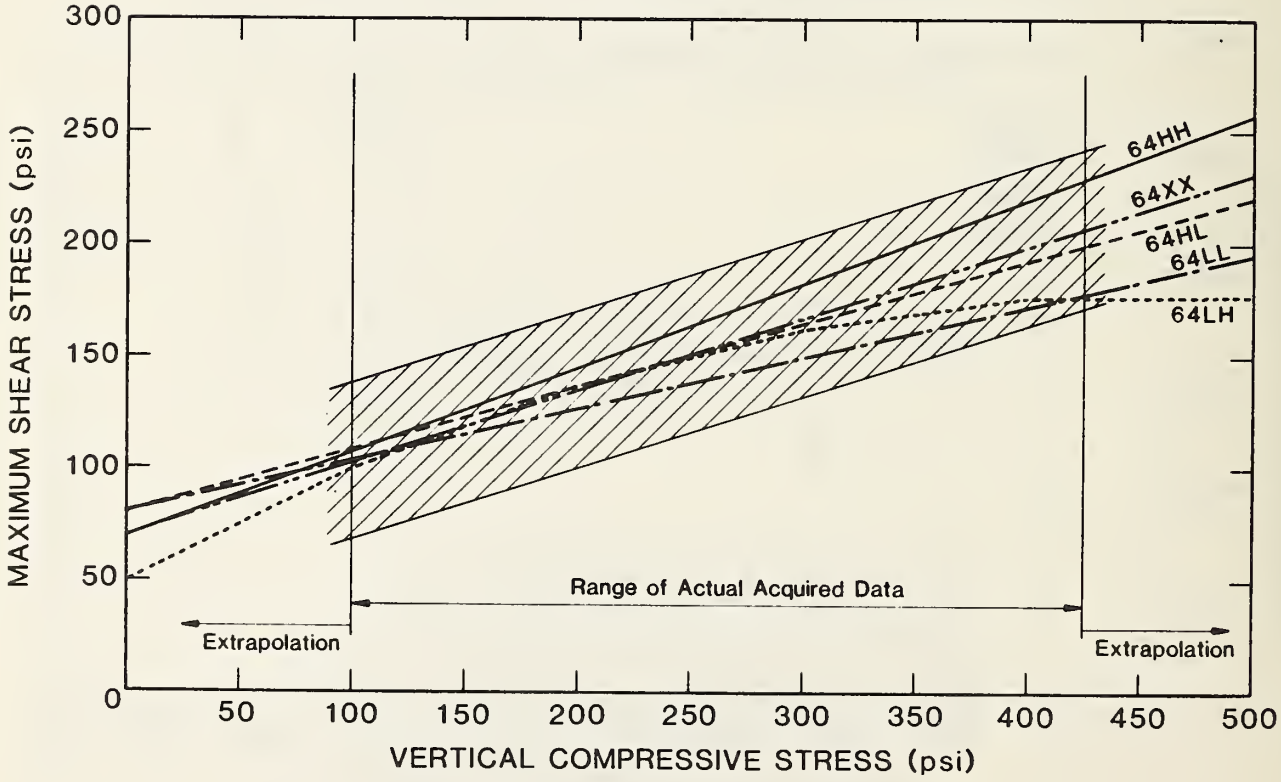


Figure 3.9 Regression analysis curves

affected by the interaction of block and mortar strengths rather than simply by each strength independently. The regression line for all of the 64 in. long walls is plotted in fig. 3.9 and it is labelled 64XX. The equation of the line is $v = (70.8 + 0.321a)$ where v is the maximum shear stress and a is the vertical compressive stress, both in units of psi. The standard error of the regression analysis (e) is 17.4 psi and parallel lines $2e$ above and below the regression line are also shown on the plot.

3.3 SHEAR STRESS-DISPLACEMENT RELATIONSHIP

The measured shear stress-displacement curves are shown in figures 3.10 through 3.15. The shear stress is the measured global in-plane lateral load divided by the wall's net cross-sectional area and the displacement is the in-plane wall displacement as measured by the top west LVDT (fig. 2.3). The curves include only the data up to a displacement just past that corresponding to the maximum shear stress.

The curves in figures 3.10 and 3.11 illustrate the effect of block strength variation for the 64 in. long walls built with high and low strength mortar, respectively. As the relationships in fig. 3.10 indicate, there is a noticeable decrease in stiffness for the low strength block walls compared to high strength block walls built with high strength mortar. However, the same trend is not observed for the walls built with low strength mortar. There does not appear to be any indication that the variation in block strength significantly affects the wall displacement at which the maximum shear is achieved.

The curves in figures 3.12 and 3.13 show the effect of mortar strength variation for the 64 in. long walls built with high and low strength block, respectively. Based on the information presented in fig. 3.12, it appears that for the walls built with high strength block a decrease in mortar strength causes a decrease in wall stiffness. A similar trend is not apparent in the data for walls built with low strength block. These relationships also do not exhibit a tendency for the mortar strength to affect the wall displacement at which the maximum shear stress occurs.

Shear stress-displacement curves are shown in figures 3.14 and 3.15 for 48 in. and 96 in. long walls, respectively. All of the wall panels whose data are shown in these two figures have high strength mortar, but have either low or high strength block. Based on the trends observed in the 64 in. wall panels, it is expected that the low strength block walls will exhibit a marked lower stiffness, but the wall displacement coincident with the maximum shear stress will be relatively unchanged. As the curves in figures 3.14 and 3.15 illustrate, the trends observed for 64 in. walls do, in fact, exist for the 48 in. and 96 in. long walls.

3.4 DIAGONAL STRAIN

The primary mode of distress in the walls is diagonal tension cracking. It seems reasonable that cracking is a function of the tensile component of the diagonal stresses or strains. It is suggested by the previous reports [1, 2] that diagonal tension strain is a reliable indicator of the onset of diagonal

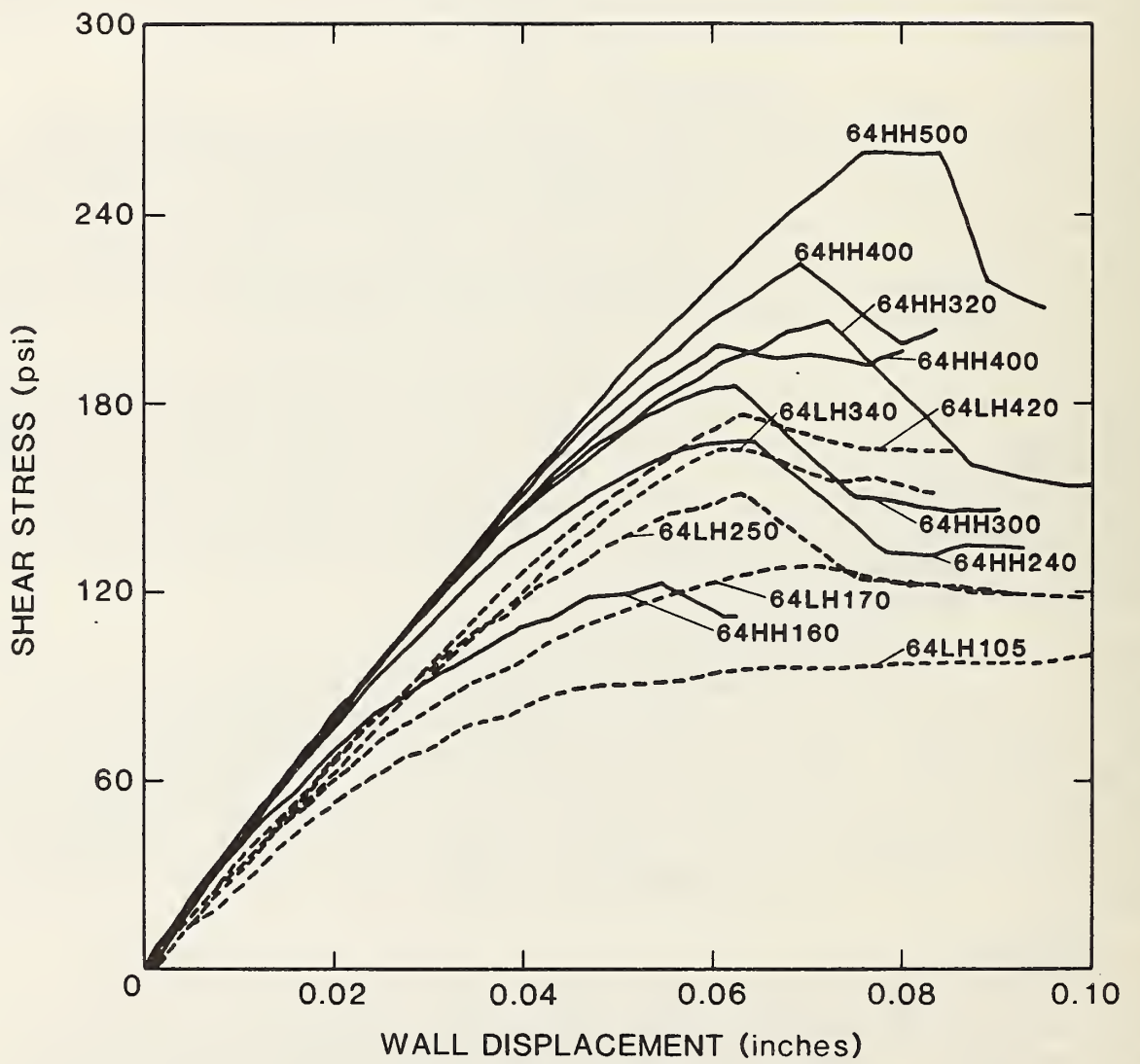


Figure 3.10 Shear stress-displacement curves (64HH & 64LH)

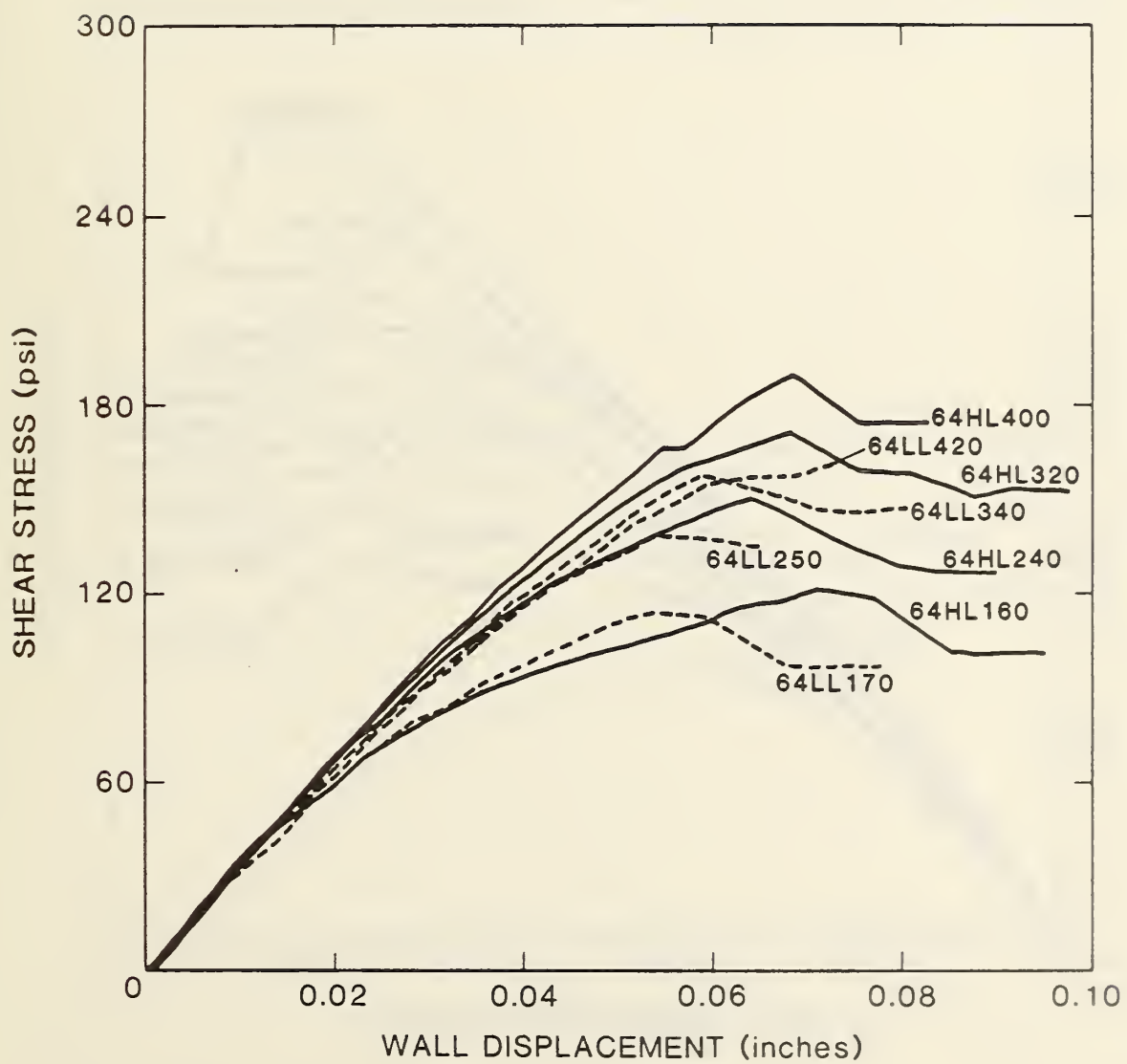


Figure 3.11 Shear stress-displacement curves (64HL & 64LL)

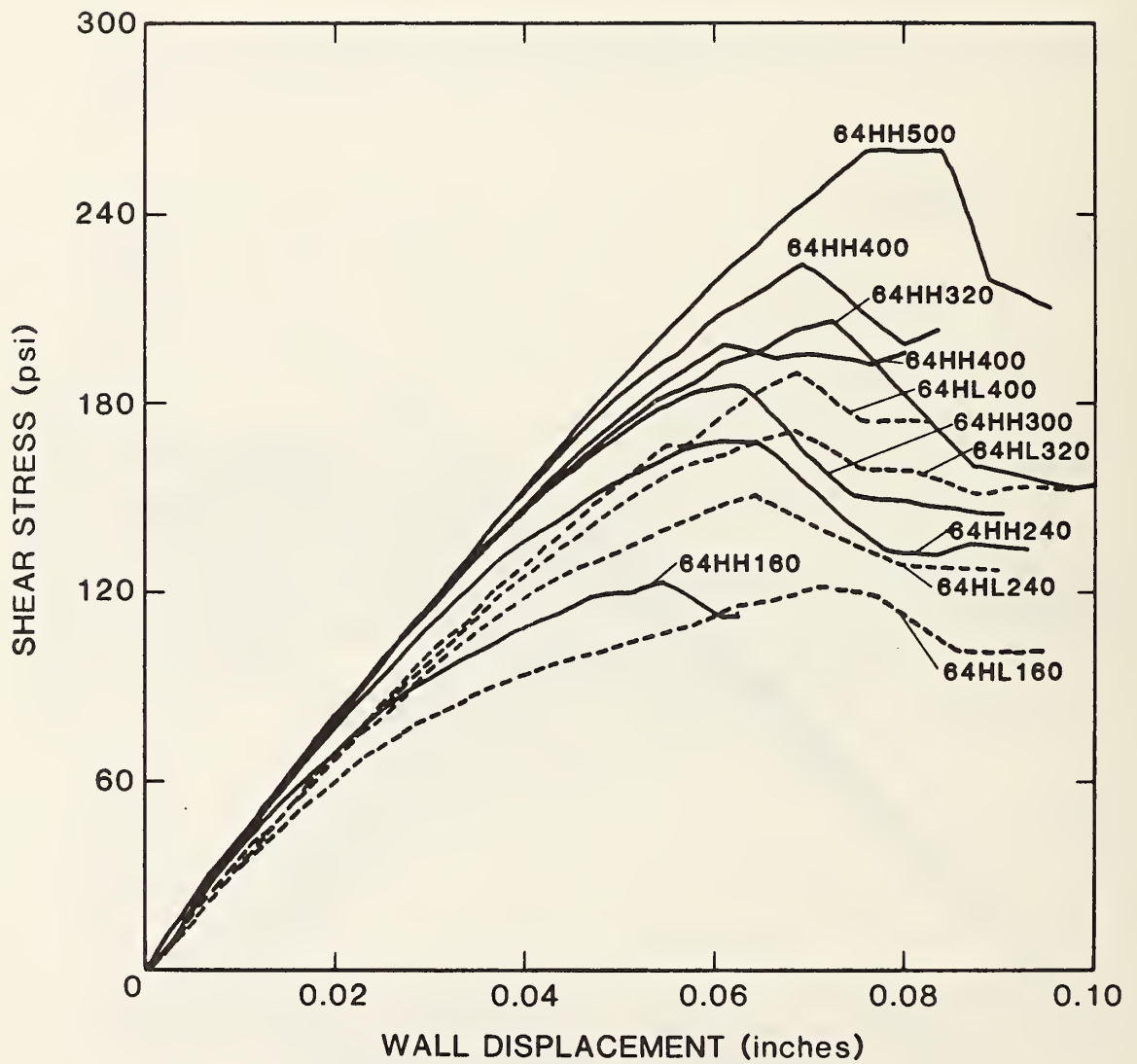


Figure 3.12 Shear stress-displacement curves (64HH & 64HL)

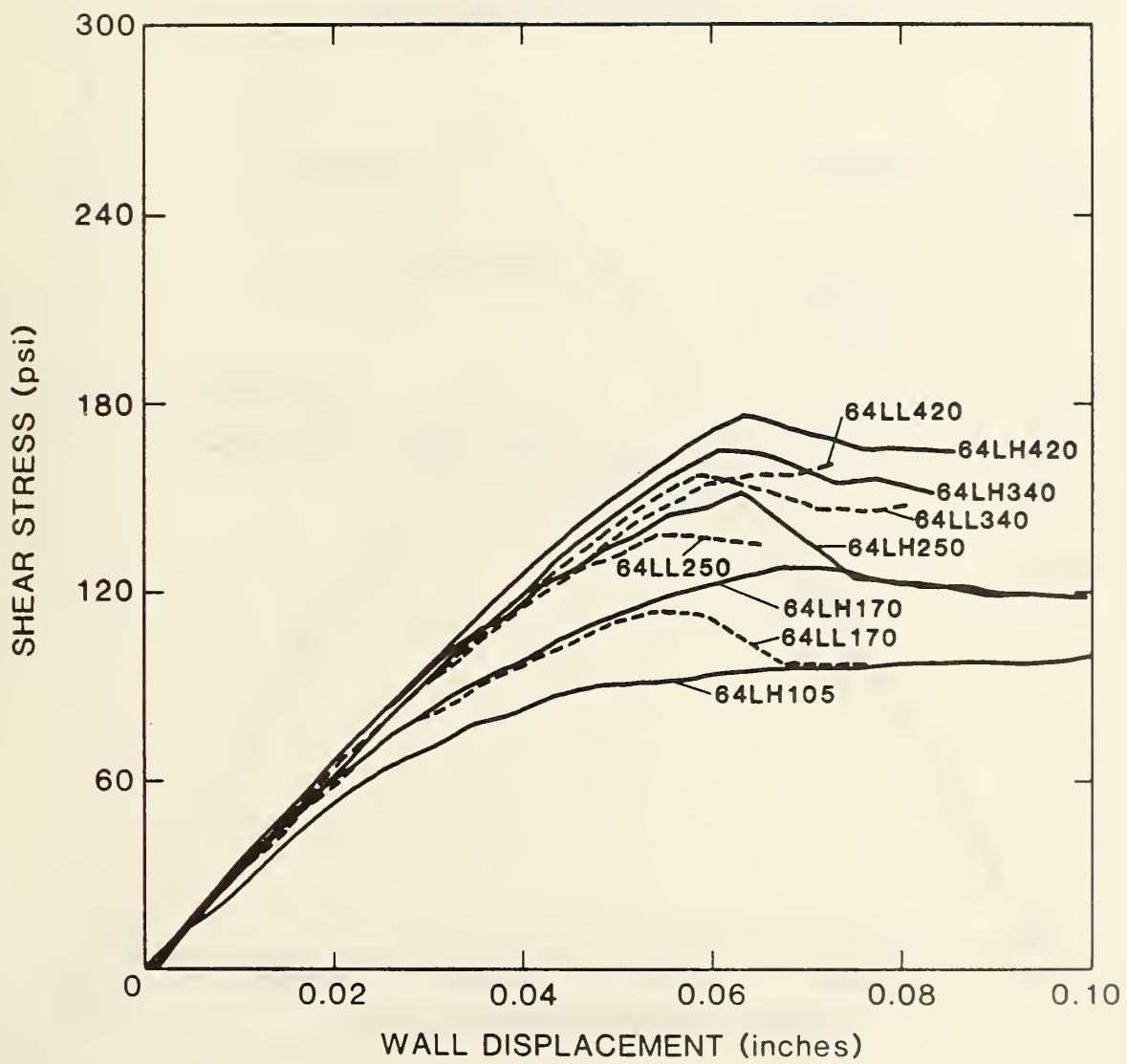


Figure 3.13 Shear stress-displacement curves (64LH & 64LI.)

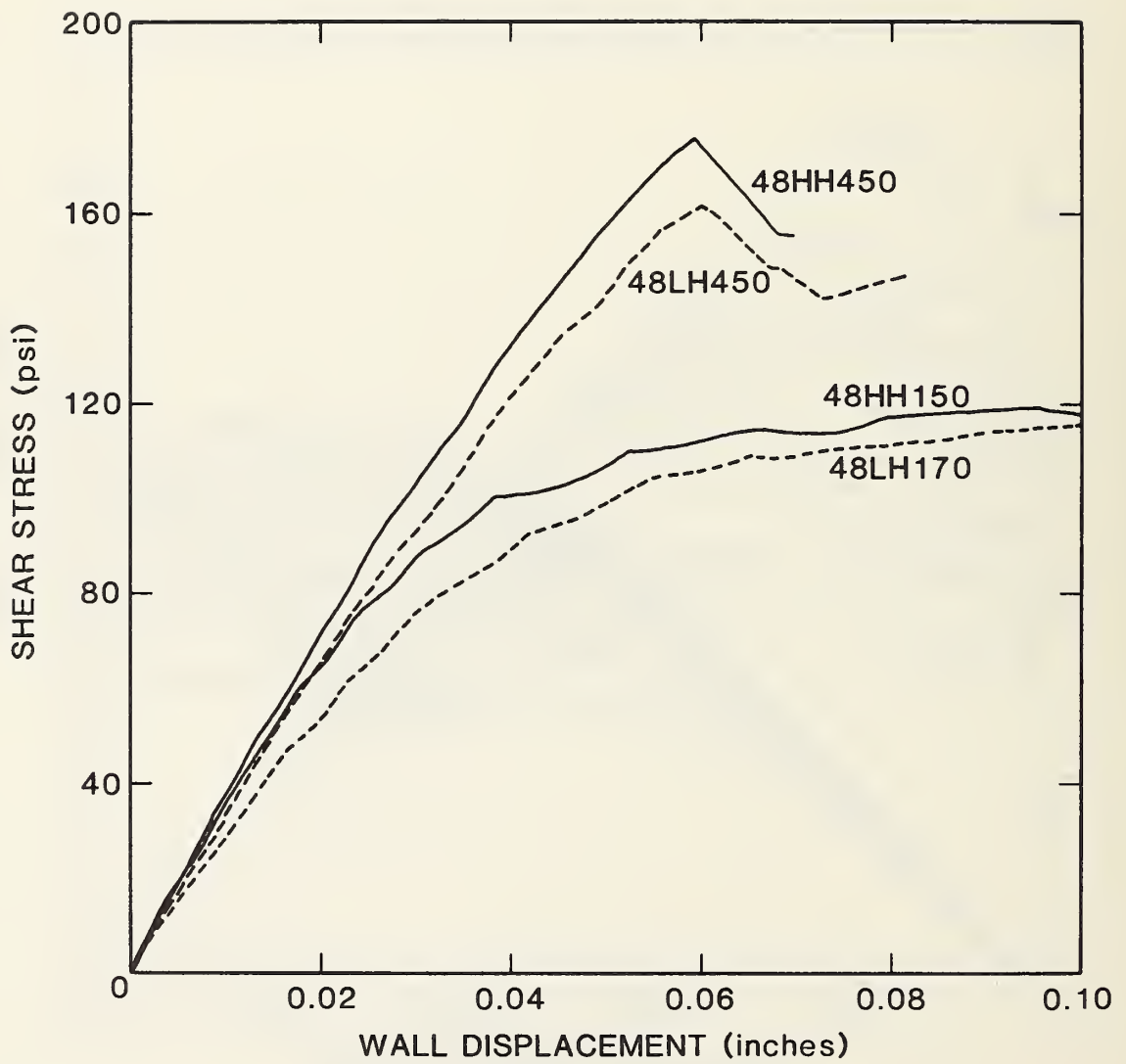


Figure 3.14 Shear stress-displacement curves (48 in.)

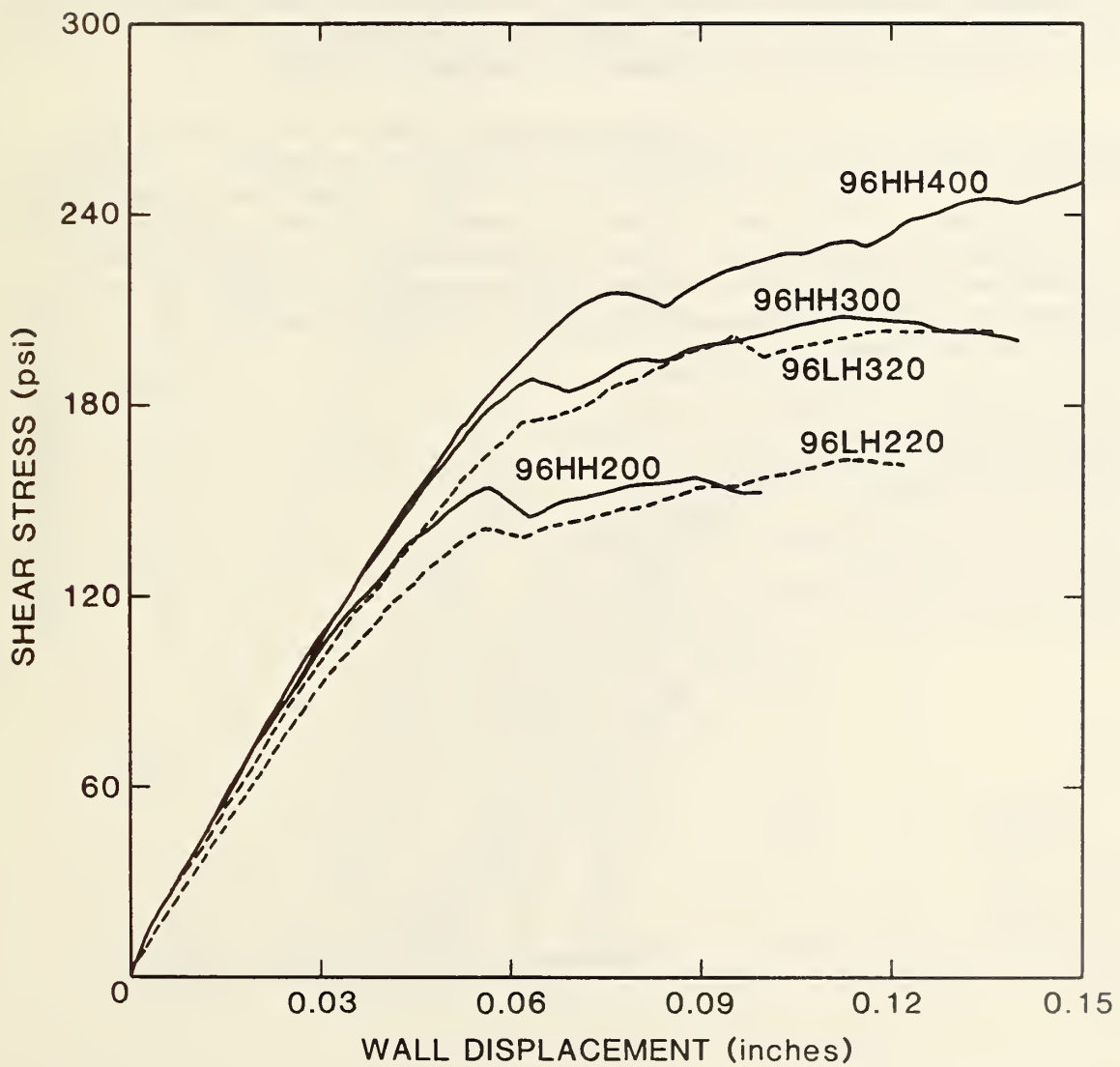


Figure 3.15 Shear stress-displacement curves (96 in.)

tension cracking and that the threshold strain at cracking is relatively unaffected by variations in vertical compressive stress and wall aspect ratio. The data from the tests reported herein also tend to suggest that the threshold strain is unaffected by variations of block and mortar strength.

The relationships between shear stress and the diagonal strain for all of the 64 in. wall panels are shown in fig. 3.16. These include data from wall panels reported previously. The diagonal strain is that computed from the displacements measured by the NL diagonal LVDT shown in fig. 2.7. With four noticeable exceptions, the curves show a definite tendency to exhibit unstable strain growth, indicative of cracking, at a common threshold strain. The exceptions are all from tests with relatively high vertical stress levels (>300 psi), but not all wall panels subjected to such vertical stresses exhibited this atypical behavior. Within the scope of this report, the exceptions will remain anomalies and possible explanations for their behavior are not discussed.

The data shown in fig. 3.16 is divided into other figures (fig. 3.17-3.20) which highlight the trends for changing block or mortar strength. The effect of block strength is shown in figures 3.17 and 3.18 for high and low strength mortar, respectively. The effect of mortar strength is shown in figures 3.19 and 3.20 for high and low strength block, respectively. The data in these figures emphasize that within the variation of parameters actually tested the threshold diagonal strain is independent of the varied parameters.

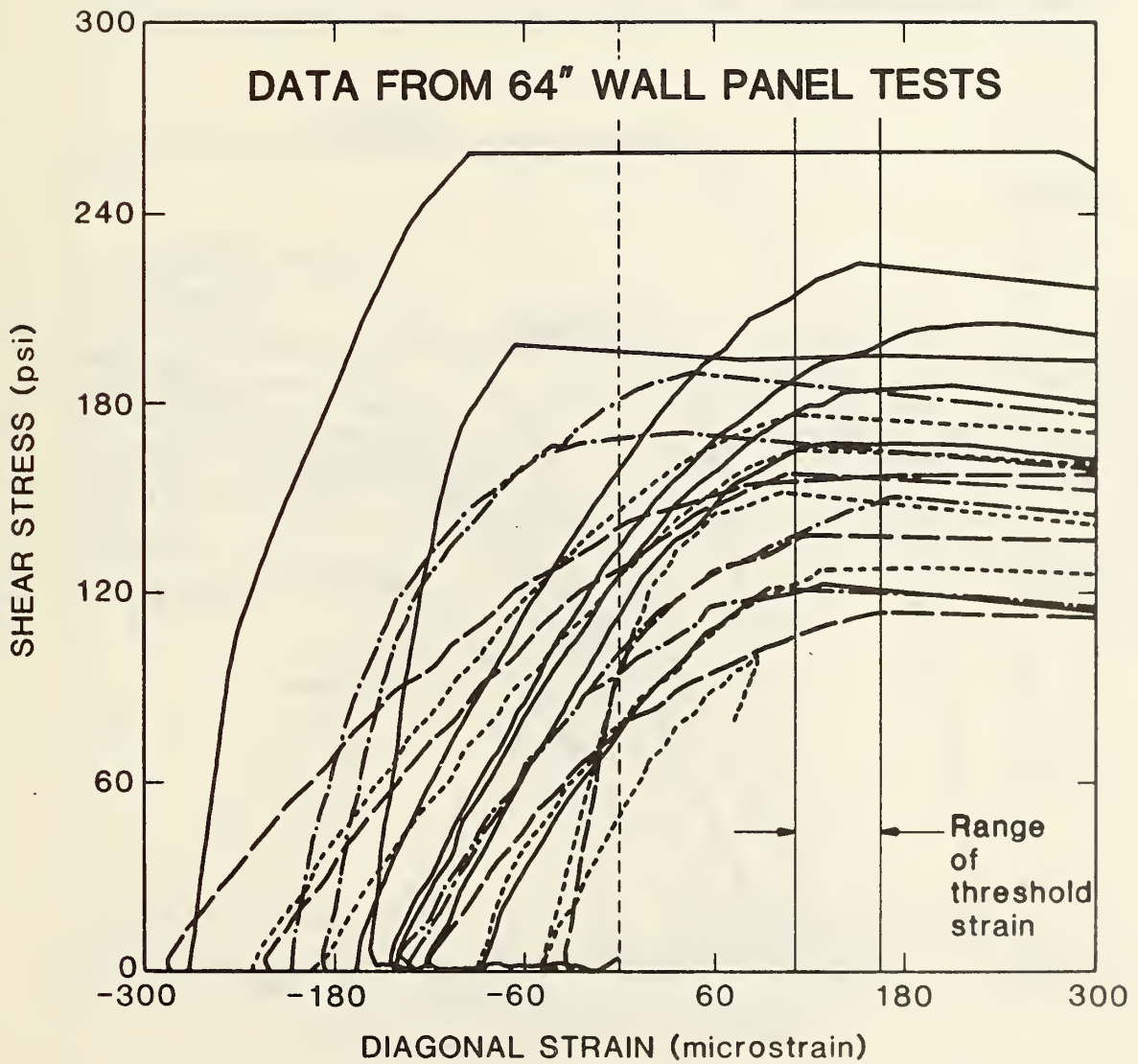


Figure 3.16 Combined shear stress-diagonal stress curves

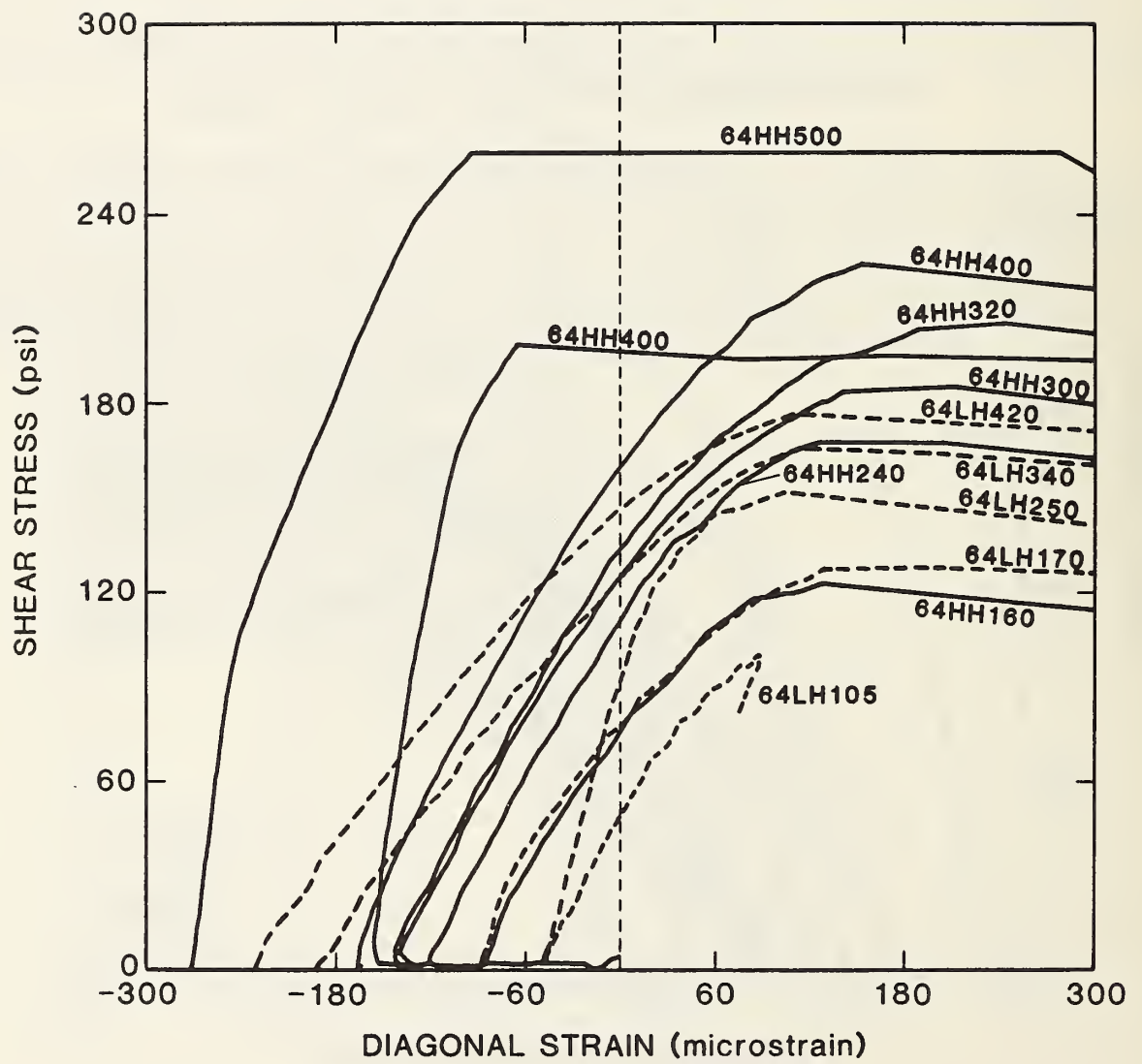


Figure 3.17 Shear stress-diagonal strain curves (64HH & 64LH)

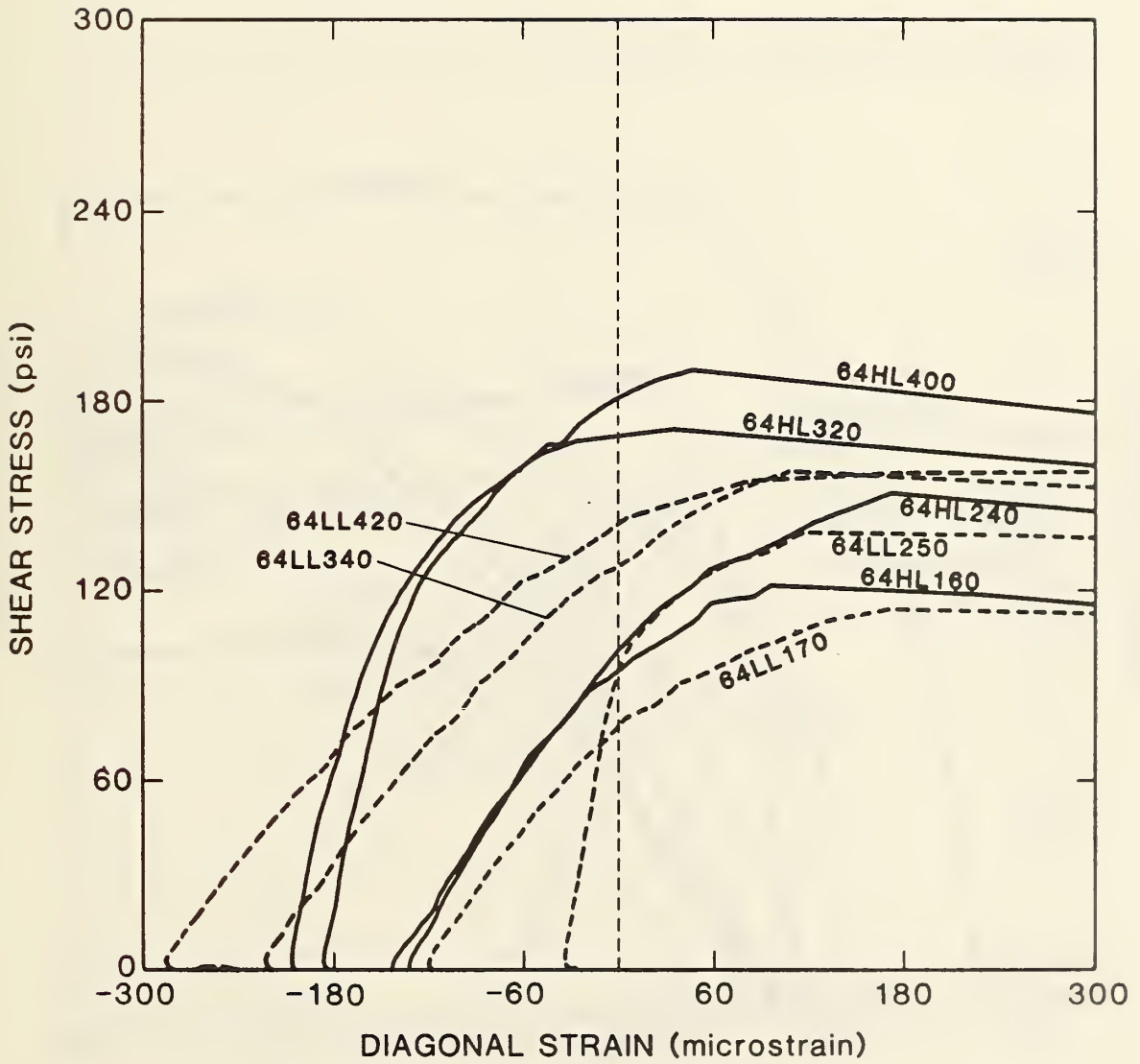


Figure 3.18 Shear stress-diagonal strain curves (64HL & 64LL)

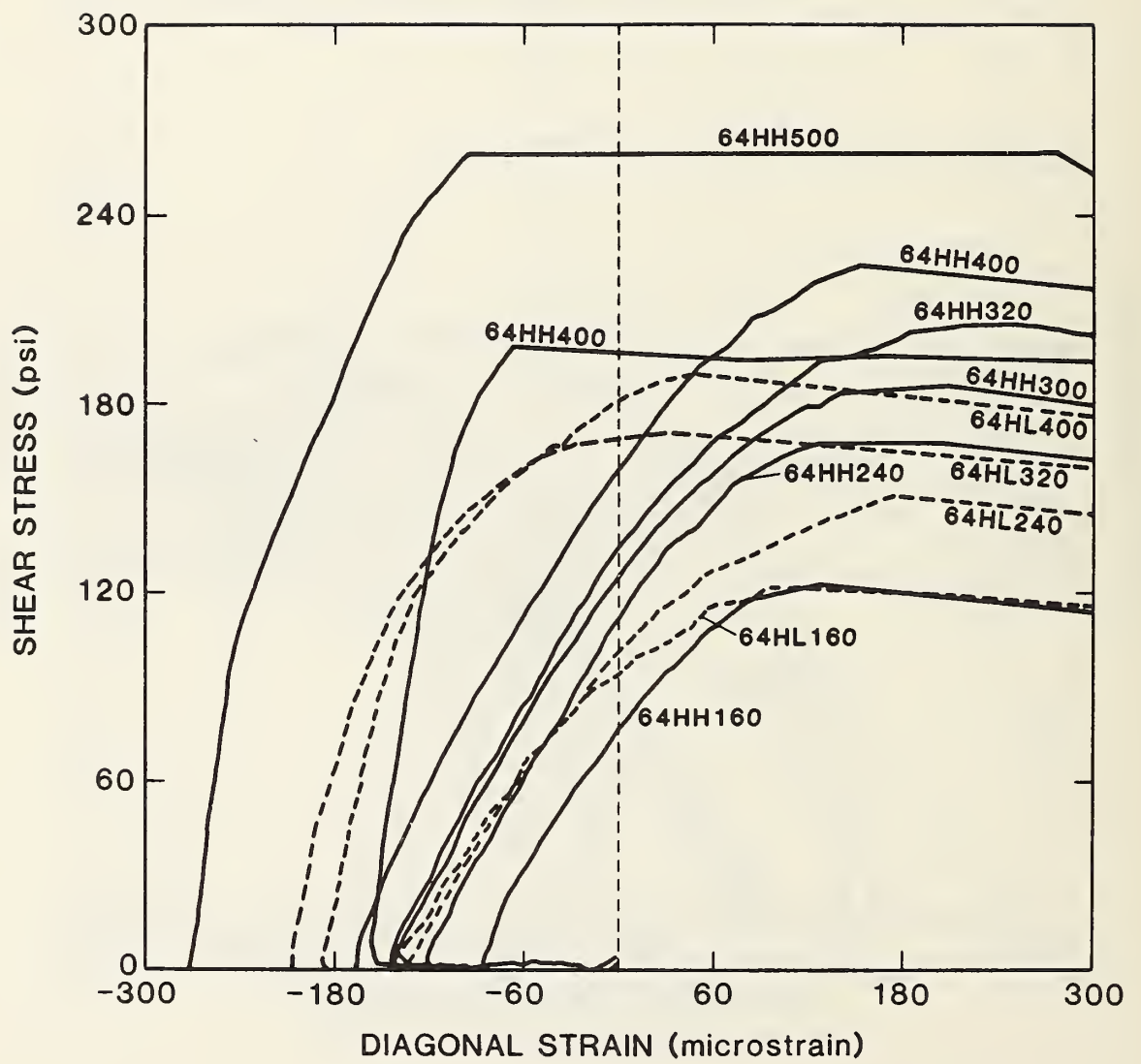


Figure 3.19 Shear stress-diagonal strain curves (64HH & 64HL)

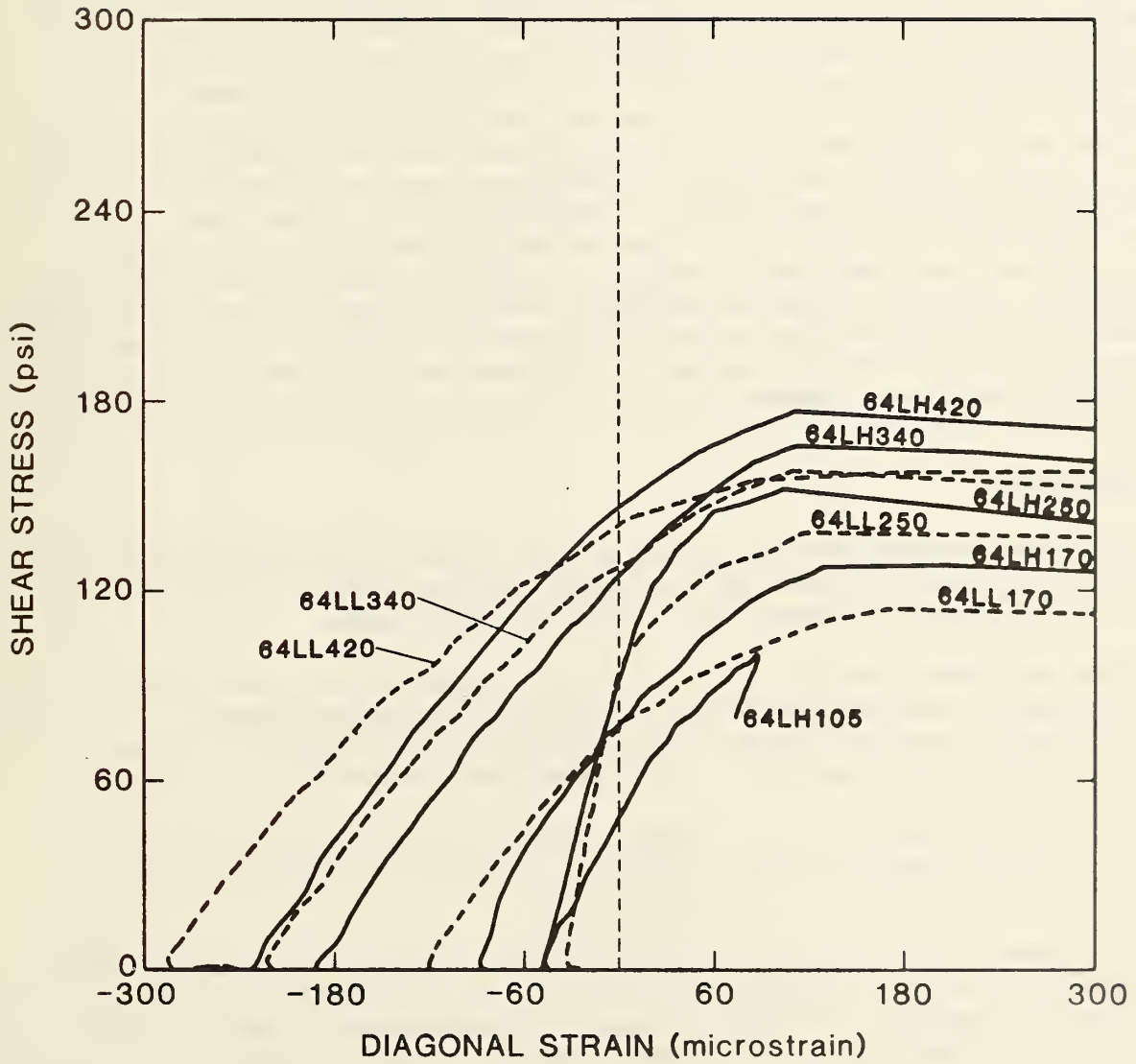


Figure 3.20 Shear stress-diagonal strain curves (64LH & 64LL)

4. SUMMARY AND CONCLUSIONS

4.1 SUMMARY

Data from seventeen wall tests were presented. The walls were built using hollow concrete block having an approximate gross area unit strength of either 1800 psi or 1300 psi and mortar proportioned as either Type S or Type N. All of the walls had a nominal height and thickness of 64 in. and 8 in., respectively. Thirteen of the walls had a length of 64 in., two of the walls were 48 in. long, and the remaining two walls were 96 in. long. The applied vertical (axial) compressive stress was varied in the test series between the approximate limits of 100 psi and 400 psi, based on the net cross-sectional area of the wall. The walls were subjected to in-plane lateral displacement in combination with a constant applied vertical compressive stress. The displacement was applied at the top of the wall while the bottom of the wall was held stationary. Rotation of the loading surface was restrained at the top and bottom of the wall, simulating a fixed-fixed boundary condition. The data indicated that for the lower levels of applied vertical stress used in the test series the influence of both block and mortar strength on the maximum shear resistance was negligible. However, as the vertical stress levels were increased the influence became significant with the expected result that higher strength block and/or mortar resulted in higher shear resistances. There also appeared to be an interaction between the effects of block and mortar strength on the shear resistance, suggesting that it may not be adequate to consider their effects independently.

4.2 CONCLUSIONS

The conclusions stated below are based solely on the data reported herein and data previously reported from this research program. The conclusions are only valid for the ranges of parameters included in these tests.

- For the lower levels of applied vertical compressive stress the influence of block and mortar strength on the maximum shear resistance was negligible. The influence of the strengths became more significant as the vertical stress was increased.
- Where block and mortar strengths influenced shear resistance, the influence appeared to be a function of their interactive effects and not solely due to one or the other.
- In general, the linear relationship between maximum shear resistance and applied vertical compressive stress was unaffected by block or mortar strength. The high strength block-low strength mortar walls were an exception and exhibited a quadratic relationship.
- The diagonal tension strain threshold at which diagonal cracking occurred was unaffected by the variation in block and mortar strength.

5. ACKNOWLEDGEMENTS

The concrete block and the mason who fabricated the walls were provided by the National Concrete Masonry Association.

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6. REFERENCES

1. Woodward, Kyle and Rankin, Frank, "Influence of Vertical Compressive Stress on Shear Resistance of Concrete Block Masonry Walls," Report No. NBSIR 84-2929, National Bureau of Standards, Gaithersburg, MD, October 1984, 53 pp.
2. Woodward, Kyle and Rankin, Frank, "Influence of Aspect Ratio on Shear Resistance of Concrete Block Masonry Walls," Report No. NBSIR 84-2993, National Bureau of Standards, Gaithersburg, MD, January 1985, 56 pp.
3. Leyendecker, Edgar V. and Cattaneo, Louis E., "Review and Refinement of ATC3-06 Tentative Seismic Provisions - Report of Technical Committee 5: Masonry," Report No. NBSIR 80-211-5, National Bureau of Standards, Washington, D.C., October 1980, 421 pp.
4. "Sampling and Testing Concrete Masonry Units," (ASTM C140-75), 1983 Annual Book of ASTM Standards, Vol. 4.05, American Society for Testing and Materials, Philadelphia, PA, pp. 117-120.
5. "Standard Specification for Ground Iron Blast-Furnance Slag for Use in Concrete and Mortar," (ASTM C989-82), 1983 Annual Book of ASTM Standards, Vol. 4.02, American Society for Testing and Materials, Philadelphia, PA, pp. 629-633.
6. "Standard Specification for Blended Hydraulic Cements," (ASTM C595-83), 1983 Annual Book of ASTM Standards, Vol. 4.02, American Society for Testing and Materials, Philadelphia, PA, pp. 359-366.
7. "Standard Specification for Mortar for Units Masonry," (ASTM C270-82), 1983 Annual Book of ASTM Standards, Vol. 4.01, American Society for Testing and Materials, Philadelphia, PA, pp. 250-254.
8. "Standard Specification for Aggregate for Masonry Mortar," (ASTM C144-81), 1983 Annual Book of ASTM Standards, Vol. 4.02, American Society for Testing and Materials, Philadelphia, PA, pp. 107-108.
9. "Standard Specification for Portland Cement," (ASTM C150-83), 1983 Annual Book of ASTM Standards, Vol. 4.01, American Society for Testing and Materials, Philadelphia, PA, pp. 156-162.
10. "Standard Specification for Hydrated Lime for Masonry Purposes," (ASTM C207-79), 1983 Annual Book of ASTM Standards, Vol. 4.01, American Society for Testing and Materials, Philadelphia, PA, pp. 221-224.
11. Woodward, Kyle and Rankin, Frank, "The NBS Tri-directional Test Facility," Report No. NBSIR 84-2879, National Bureau of Standards, Washington, D.C., May 1984, 43 pp.

Appendix A. OVERVIEW OF MASONRY RESEARCH PROGRAM

The principal objective of the overall program of research is to define the shear capacity and behavior of shear-dominated masonry walls. The prediction of shear capacity and behavior of masonry has been identified as an area in which there is serious deficiency of supporting research. The NBS/BSSC review committee for the ATC3-06 masonry design provisions [3] suggested that research was needed to substantiate and improve the current design recommendations for shear capacity.

The main variables which are to be investigated in the NBS masonry research program are axial compressive stress, aspect ratio (wall length-to-height), masonry type, mortar type, grout, vertical and horizontal reinforcement, out-of-plane loadings, and loading history. Analytical studies are coordinated with the experimental investigations so that a predictive model can be developed for defining the shear capacity and behavior of a masonry wall. The predictive model will lead to improved design standards, but in the interim the experimental test results will aid in substantiating and improving the current design provisions for shear in masonry walls.

Appendix B. MATERIALS

All materials used in the wall panel construction and associated prisms were commercially available and were representative of those commonly used in building construction.

B.1 CONCRETE MASONRY UNITS

Two concrete masonry unit shapes were used in the construction of the wall panels and prisms:

1. 8 in. x 8 in. x 16 in., 2 core hollow stretcher block.
2. 8 in. x 8 in. x 16 in., 2 core hollow kerfed corner block with a steel sash groove at one end.

The dimensions represent nominal sizes. Typical measured dimensions and physical characteristics of the units are presented in table B.1. The measurements were made in accordance with the procedures set forth in ASTM C140 [4]. The units are illustrated in fig. B.1. The half blocks at each end of alternating wall courses were made by sawing kerfed corner blocks in half through the kerf. Both halves produced by this procedure were used in the wall panels.

All of the concrete masonry units used in the wall panels and prisms were manufactured on the same day by a commercial block manufacturer. The mixture proportions were set to produce units having ultimate gross area compressive strengths of either 1000 psi or 2000 psi. The mixture proportions were:

<u>1000 psi unit</u>	<u>2000 psi unit</u>	
1950 lbs	1950 lbs	lightweight expanded shale aggregate
1250 lbs	1250 lbs	sand
200 lbs	260 lbs	portland cement
190 lbs	190 lbs	NEWCEM

NEWCEM is the proprietary name for a very finely ground water granulated blast furnace slag manufactured by Atlantic Cement Co., Inc. and is used as a partial replacement for portland cement. It meets the requirements of ASTM C989, grade 120 [5] and when blended within the range of 25 to 65 percent with portland cement, meets the requirements of ASTM C595 [6]. The preceding description of NEWCEM is presented only for purposes of information and is not an endorsement of the proprietary product. The mixture used in producing the 1000 psi and 2000 psi units made 118 units with 3.3 lbs of cementitious materials per unit and 115 units with 3.9 lbs of cementitious materials per unit, respectively.

B.2 MORTAR

Two types of mortar were used in constructing the wall panels and prisms. One type of mortar was a portland cement-lime mortar that was proportioned within the limits of a Type S mortar according to the specifications of ASTM C270 [7].

The other type of mortar was also a portland cement-lime mortar, but proportioned within the limits of a Type N mortar. The materials used in the mortar were:

1. Sand - A natural bank sand that was dug locally with its primary use being for masonry mortar. Sieve analyses were performed on the sand upon delivery. The analyses were done according to the specifications in ASTM C144 [8] and the results appear in table B.2. The fineness modulus was 1.57.
2. Portland cement - A commercially available, bagged, 94 lbs per bag, Type I portland cement identified as meeting the specifications of ASTM C150 [9].
3. Lime - A commercially available bagged, 50 lbs per bag, hydrated lime, Type S, identified as meeting the specifications of ASTM C207 [10].

The Type S mortar was proportioned 1:3/8:4 with 1 part by volume of cement, 3/8 part by volume of lime, and 4 parts by volume of sand. The Type N mortar was proportioned 1:1:5 with 1 part by volume of cement, 1 part by volume of lime, and 5 parts by volume of sand. The parts were mixed in a typical motorized mortar mixer (fixed horizontal drum with rotating blades) for a period of not less than 3 minutes after all cement, lime, sand, and most of the water were added. Finally, small amounts of water were added to produce mortar of a consistency acceptable to the mason. Retempering of the mortar, if required, was permitted only once per batch.

Table B.1 Dimensions and Properties of Concrete Masonry Units

	1000 psi Unit *		2000 psi Unit **	
	Hollow Stretcher	Hollow Corner/Sash Groove	Hollow Stretcher	Hollow Corner/Sash Groove
Width (in.)	7.65	7.64	7.63	7.63
Height (in.)	7.56	7.56	7.59	7.57
Length (in.)	15.66	15.66	15.62	15.64
Minimum Face Shell Thickness (in.)	1.30	1.29	1.30	1.30
Gross Area (in. ²)	119.8	119.6	119.2	119.3
Net Solid Area (in. ²)	60.4	66.1	61.5	67.0
Gross Ultimate Compressive Strength (psi)	1304	1398	1813	1795
Density (lb/ft ³)	98.1	96.8	102.4	104.5
Absorption (lb/ft ³)	14.0	13.7	10.8	10.2

* Average of measurements from 9 units of each type.

** Average of measurements from 6 units of each type.

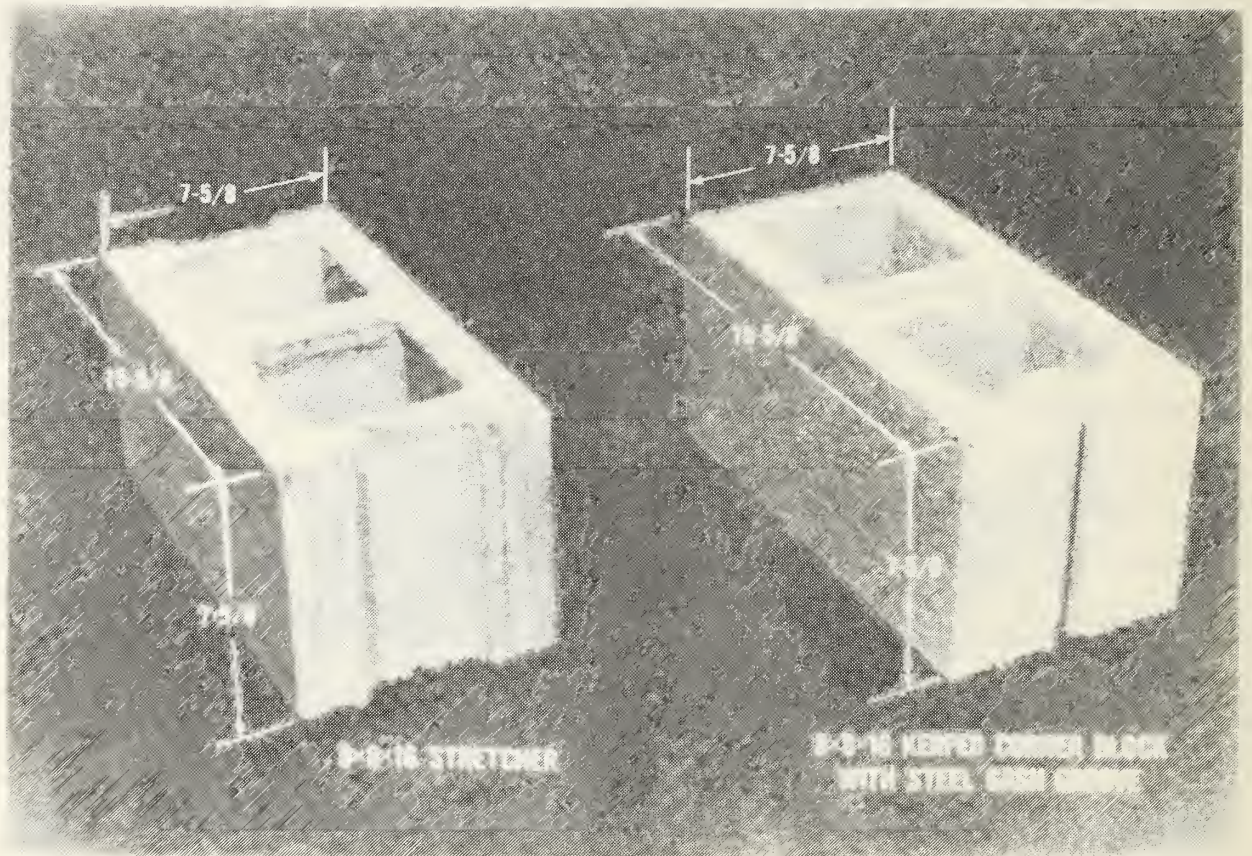


Figure B.1 Concrete Masonry Units

Table B.2 Masonry Sand Sieve Analysis*

Screen Size Number	Cumulative Percent Retained
4	0.1
8	0.6
16	2.0
30	11.0
50	59.9
100	86.6
100+

Total 157 ÷ 100 = 1.57 Fineness Modulus

*Average of three samples taken upon delivery of sand.

Appendix C. WALL PANEL DESCRIPTION

As companions to each wall, six mortar cubes (2 in. x 2 in. x 2 in.) were made and three prisms were fabricated. The companion specimens were tested to provide information on mortar compressive strength and wall panel compressive strength.

C.1 WALL PANEL FABRICATION

The wall panels were constructed in running bond with 50 percent overlap of block in alternate wall courses. The wall panels had overall nominal dimensions of 64 in. in height, 8 in. in thickness, and either 96 in., 64 in., or 48 in. in length. The wall panels (and prisms) were constructed by an experienced mason using techniques representative of good workmanship. The wall panels were fabricated in a controlled environment laboratory from materials stored in the same environment for a least 30 days. The temperature and relative humidity of the laboratory were maintained at approximately 73°F and 50 percent, respectively.

The bottom course of block was laid on a steel beam (channel) section without bedding mortar. The steel beam and first course were then leveled using shims as necessary. The first block laid in the bottom course was a whole kerf unit with no head joint mortar. Head joints were subsequently formed by buttering the end of the next block to be laid with mortar. The head joints were "shoved" joints with no closure units or backfilling of head joints. The mortar bed joints were formed by placing mortar along the face shells of the previously laid course of blocks. No mortar was placed on the cross-webs except for the end cross-webs. Each course was laid to maintain a course height of 8 in. The level of each course was fixed by a level string spanning between two vertically plumb posts. The end blocks were plumb using a 4 foot level to maintain plumb end surfaces of the wall panel. All joints were struck flush with a trowel, but not tooled.

C.2 PRISM FABRICATION

Prisms were made along with most of the wall panels using mortar from the same batch as was used for the wall panel. Each prism was made by stack bonding three stretcher units. The mortar bedding between the blocks was either face shell only or full area bedding. Within each group of prisms the bedding was the same. The mason used a 4 foot level to maintain the level of each block and to plumb the prism. The ultimate compressive strength of the prisms was determined by testing the prisms in uniaxial testing machine having a total capacity of 400,000 pounds force. A spherically seated upper bearing block covered the entire bearing surface of the prisms. The load on the prism was applied at any convenient rate for the first 40,000 pounds force while the remaining load was applied at a rate of 40,000 pounds per minute until failure occurred. The maximum load sustained by the prism was used in computing the ultimate compressive stress.

Appendix D. WALL PANEL TESTS: SETUP AND PLACEMENT

D.1 TEST SETUP

The test setup (fig. 2.2) is the NBS Tri-directional Test Facility (NBS/TTF), a permanent loading apparatus designed to test building components using three-dimensional loading histories. The NBS/TTF is described in a separate report [11], but for purposes of completeness a brief summary is presented in this section.

The NBS/TTF is a computer-controlled loading apparatus which can apply forces/displacements in all six degrees of freedom at one end of a test specimen. The other end of the specimen is fixed. The six degrees of freedom are the translations and rotations in and about three orthogonal axes. The application of such actions is accomplished by seven closed-loop, servo-controlled hydraulic actuators which receive their instructions by means of computer generated commands. The major components of the NBS/TTF are shown in fig. 2.2. The reaction system is composed of the structural tie-down floor and two vertical buttresses. The load distribution system consists of the two x-shaped steel crossheads, one at the bottom and the other at the top of the test specimen. The load application system is made up of the seven hydraulic actuators. The control system is not visible in the figure, but includes the servo-control electronics, the data acquisition equipment, and a minicomputer.

D.2 WALL PANEL PLACEMENT

The wall panels were handled by attaching a carrying harness to the panel (fig. D.1). The harness had attachment points for lifting the wall and a clamping arrangement which held the harness against the ends of the panel. The overhead crane was used to place the wall panel in the NBS/TTF and a special device was fabricated which permitted the wall to be placed under the upper crosshead. The special device was a large welded assembly in the shape of a "C" which permitted the crane hook to be centered above the wall panel without interfering with the upper crosshead (fig. D.2).

The walls were set in place using mechanical stops which fixed the walls in their horizontal position. The walls were aligned vertically using small wedges set at four places under the face shells of the walls. The walls were fastened to the lower crosshead first, using an epoxy mortar along the bottom face shells and end cross-webs. The upper crosshead was then lowered onto the wall whose top face shell and end cross-webs were also mortared with the epoxy mortar. A small vertical compressive load (1,000 to 2,000 pounds) was applied to the wall to ensure contact between the wall and epoxy mortar. The upper crosshead was locked in position and the epoxy mortar was allowed to cure at least 16 hours before testing the wall.

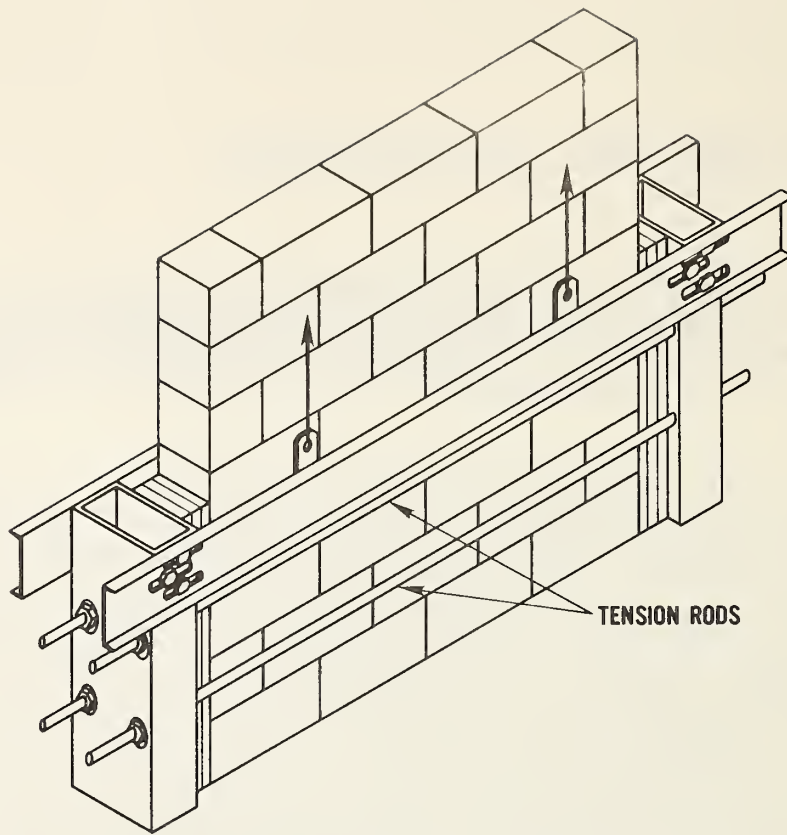


Figure D.1 Wall panel transport harness

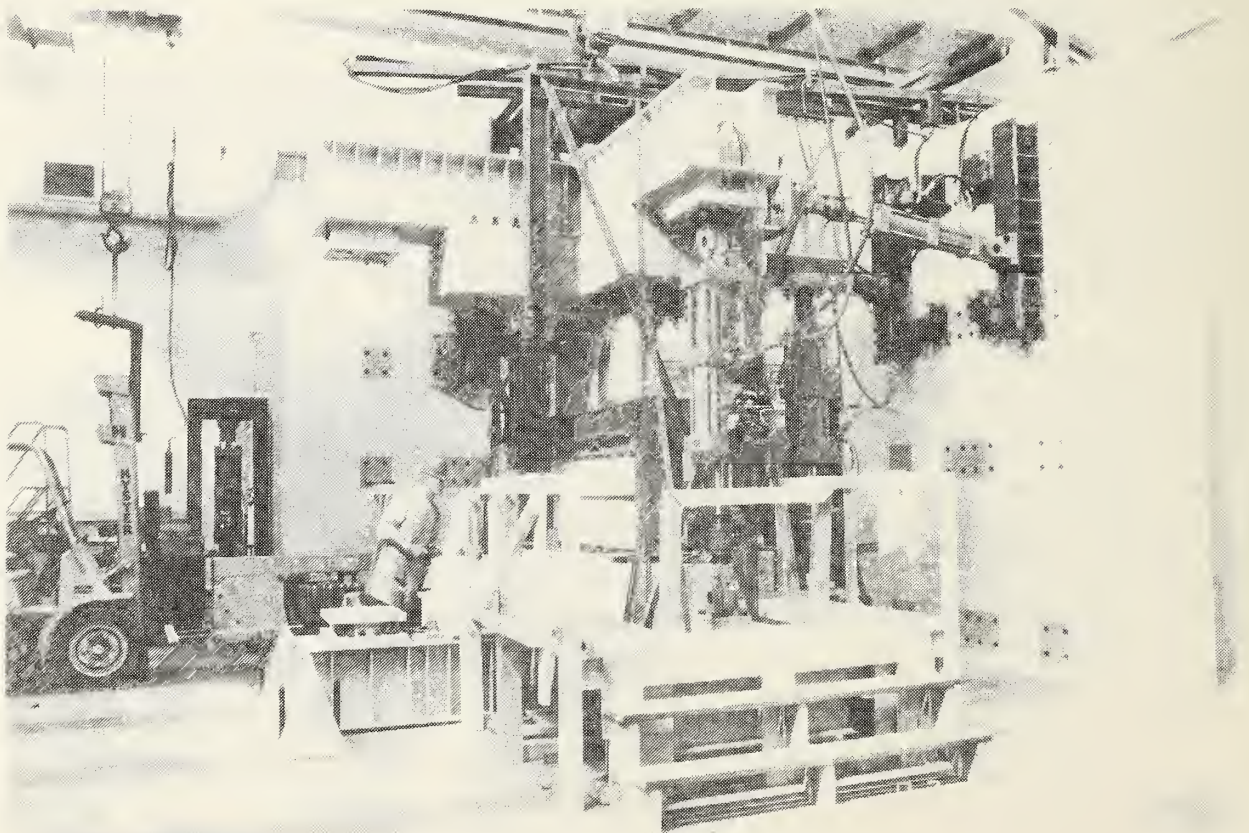


Figure D.2 Placement of a wall panel

Appendix E. WALL PANEL LOAD-DISPLACEMENT CURVES

The load-displacement curves describe the complete loading (displacement) history for each wall test and provide a primary indicator of wall behavior. The load used in developing the curves is the horizontal load acting in the plane of the wall as measured by the hydraulic actuator load transducers. The load is referred to as the global in-plane load. The displacement used in the curve is the horizontal displacement of the upper crosshead in the plane of the wall (fig. 2.9). This displacement is referred to as the global in-plane displacement to differentiate it from the in-plane wall displacement measured by the horizontal LVDT's mounted to the wall (fig. 2.3). The global in-plane displacement (GID) is determined by the displacement transducers in the hydraulic actuators. The GID and the wall displacement measured directly by LVDT's are not necessarily the same. The GID is affected by total apparatus displacement while the direct LVDT displacement more nearly measures an absolute in-plane displacement of the wall. However, the direct LVDT displacement can be strongly affected by the breakup of a wall after cracking. Spalling and splitting in the region near a wall LVDT can produce large distortions in the apparent displacement. In general, the GID is a consistent measure of displacement which is unaffected by a local wall distortions and, as a result, is best for general comparisons between tests.

The crack pattern maps reflect the observations of wall cracking at selected points during the tests. The crack patterns provide useful information on the physical response of a wall to an imposed loading history. The patterns serve as a guide to identifying regions of high stress, general stress flow, and physical load resisting mechanisms. It is, of course, desirable that the points during a test at which crack patterns are recorded be identified on the load-displacement curves. This is accomplished by using an identifying symbol which marks a location on the load-displacement curve corresponding to an associated crack pattern on the map. The symbols are capital letters starting with the letter A. Thus, points on the load-displacement curve identified by the symbols are coincident with the crack pattern also associated with that symbol. Symbol A does not refer to the same point on every load-displacement curve.

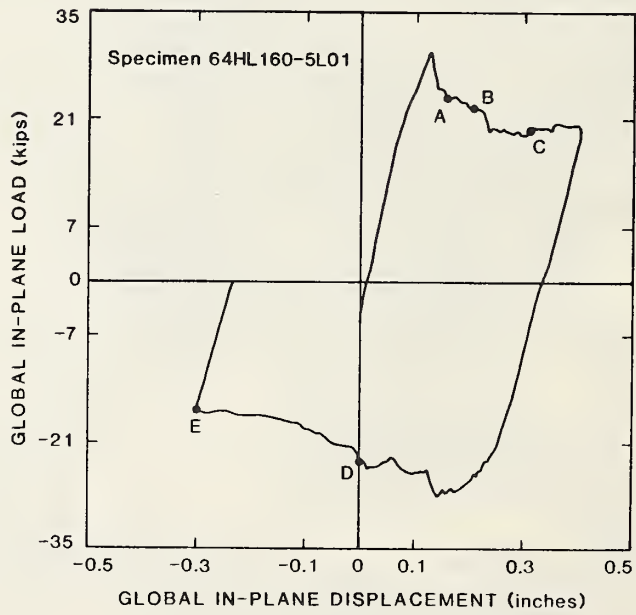
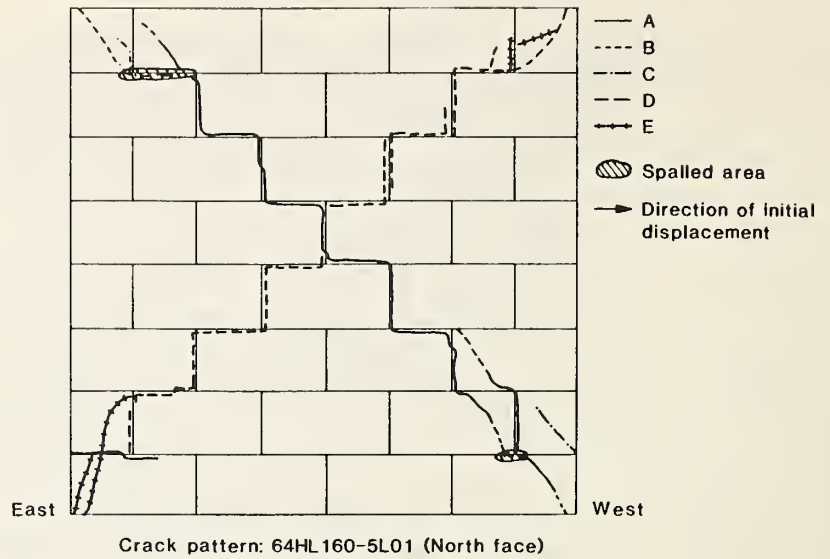


Figure E.1 Crack pattern and load-displacement curve for specimen 64HL160

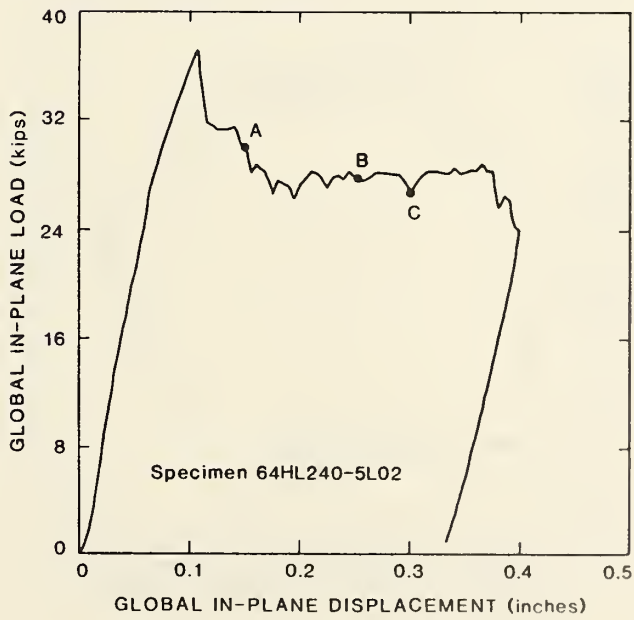
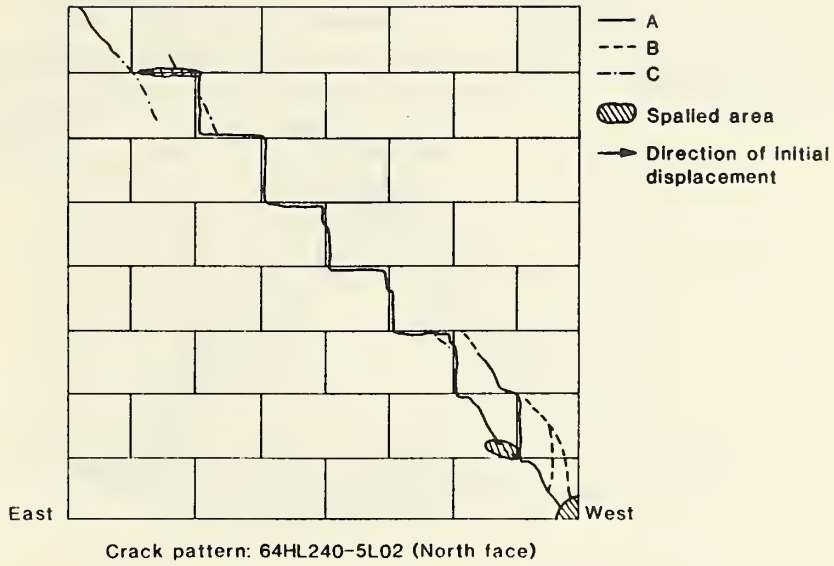


Figure E.2 Crack pattern and load-displacement curve for specimen 64HL240

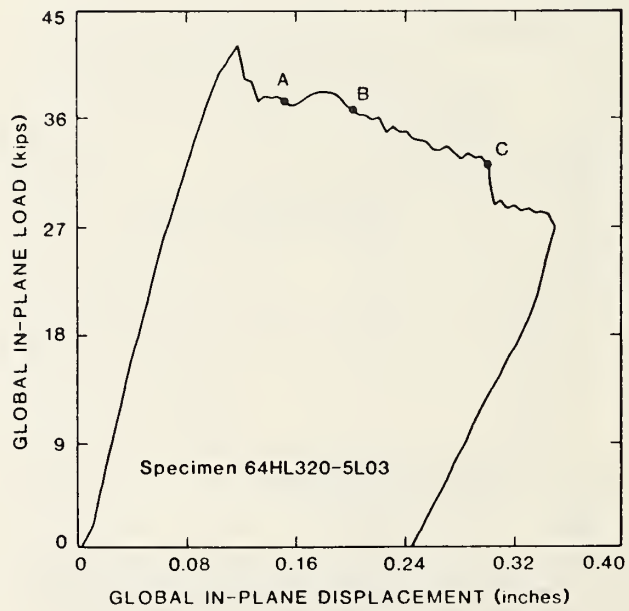
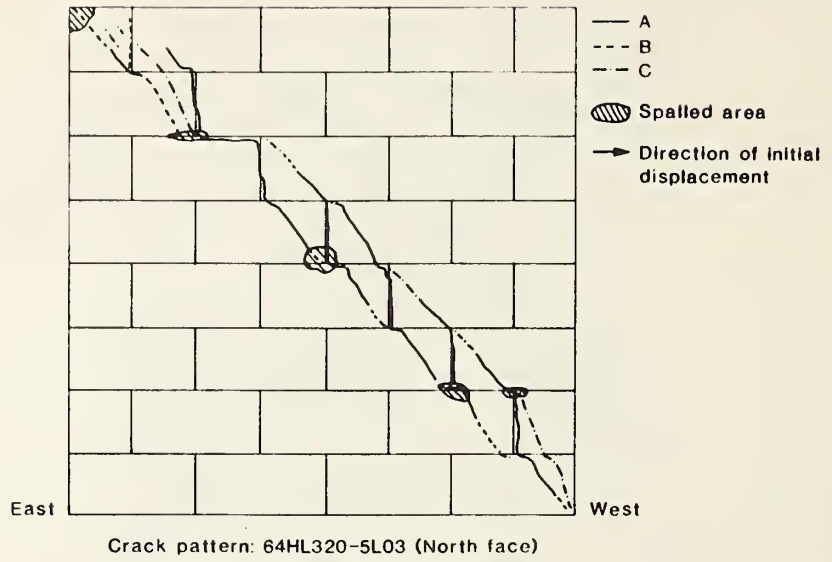


Figure E.3 Crack pattern and load-displacement curve for specimen 64HL320

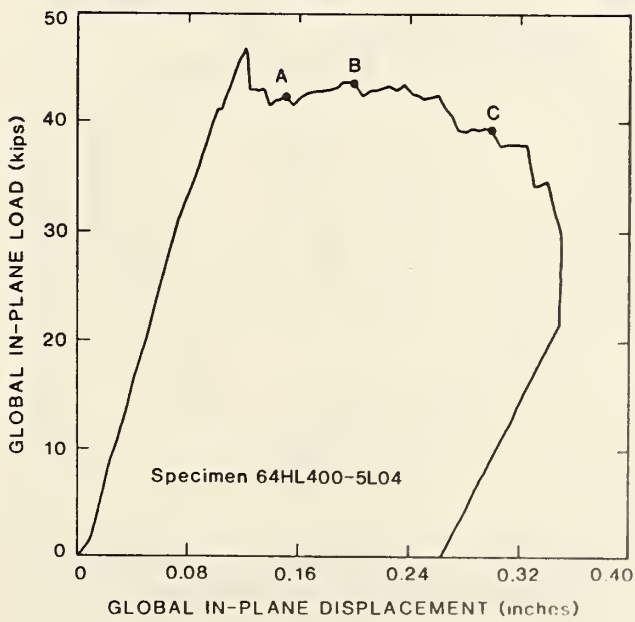
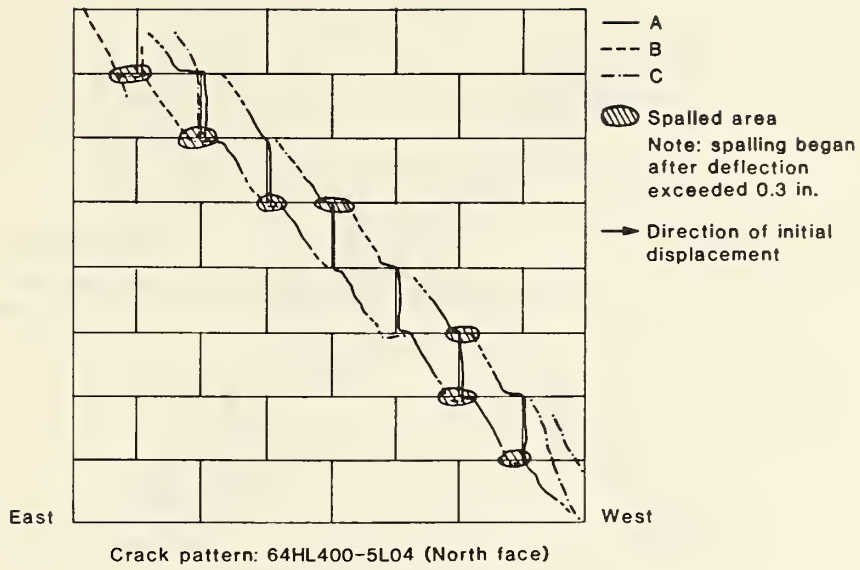


Figure E.4 Crack pattern and load-displacement curve for Specimen 64HL400

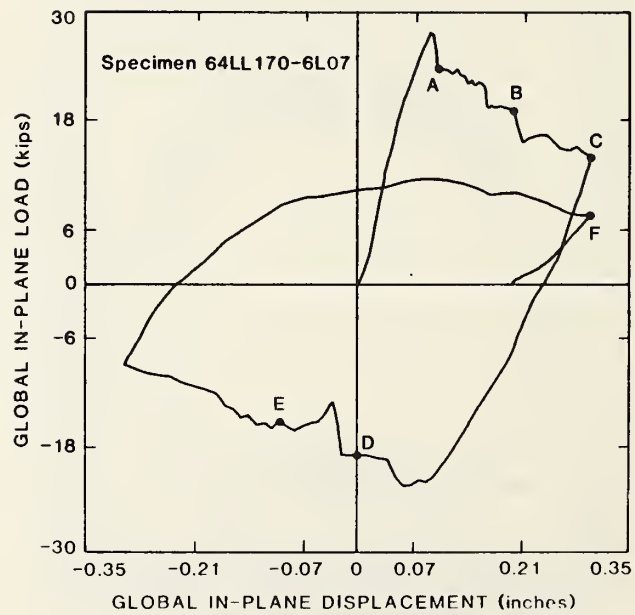
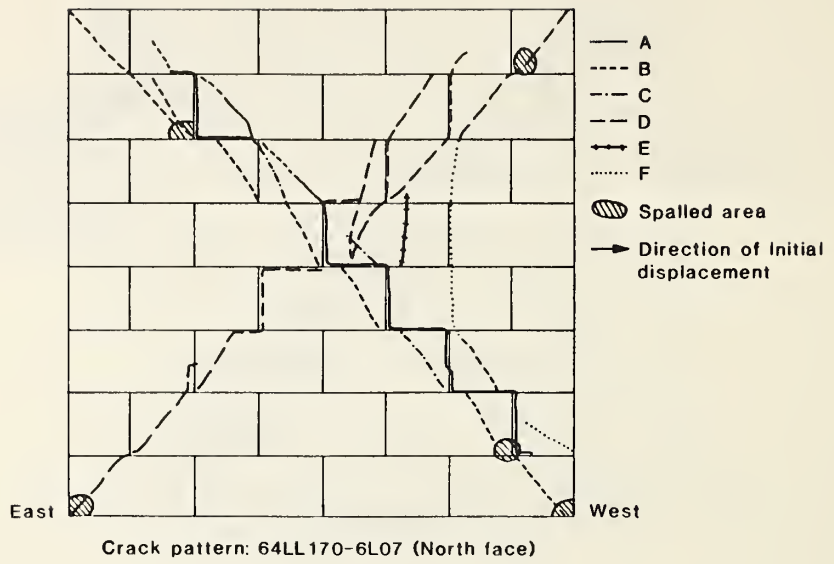


Figure E.5 Crack pattern and load-displacement curve for specimen 64LL170

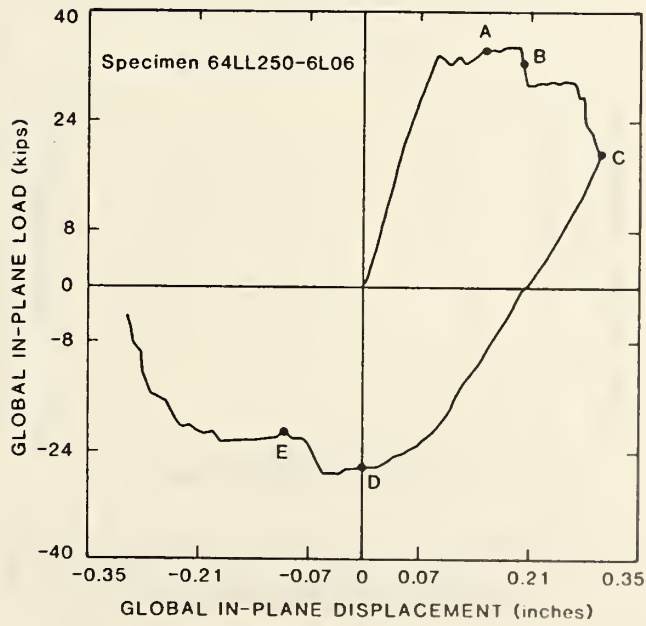
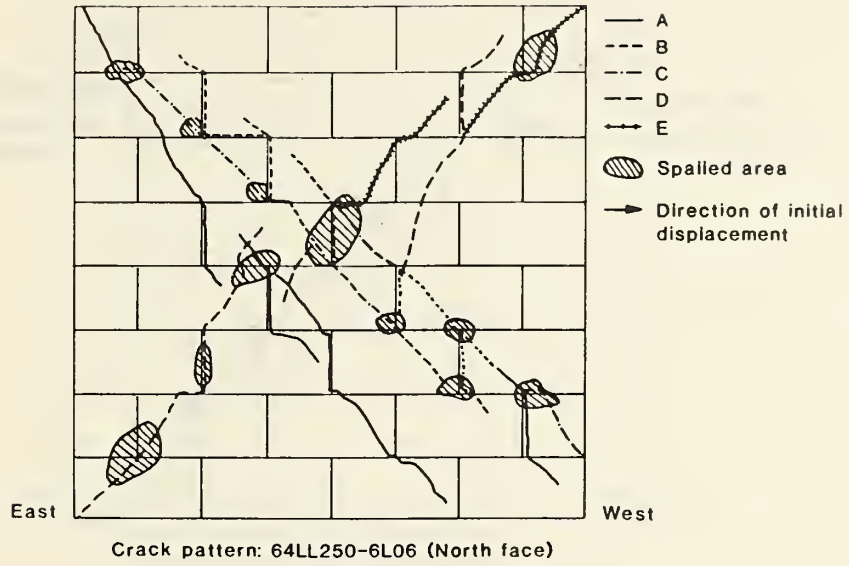


Figure E.6 Crack pattern and load-displacement curve for specimen 64LL250

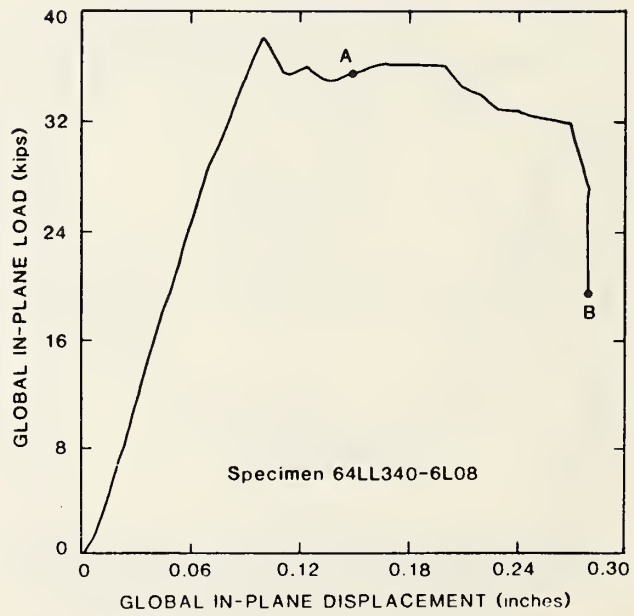
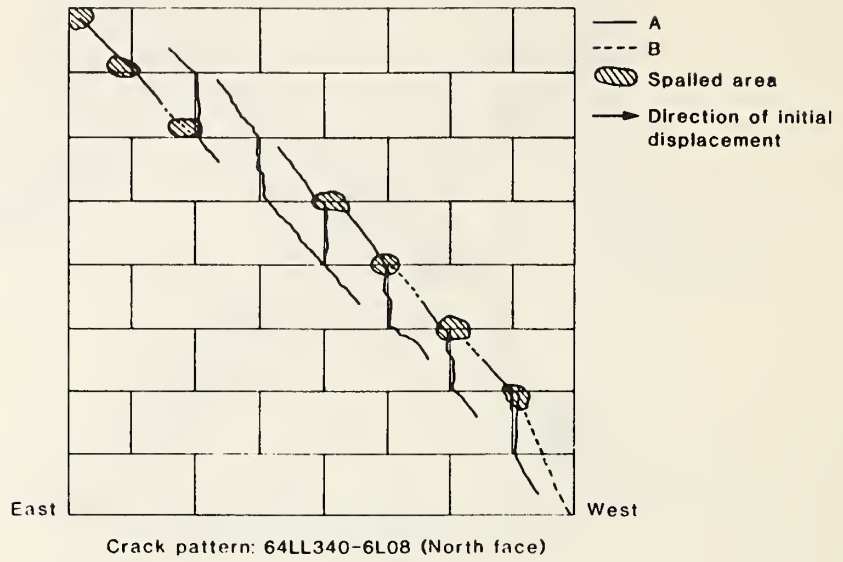


Figure E.7 Crack pattern and load-displacement curve for specimen 64LL340

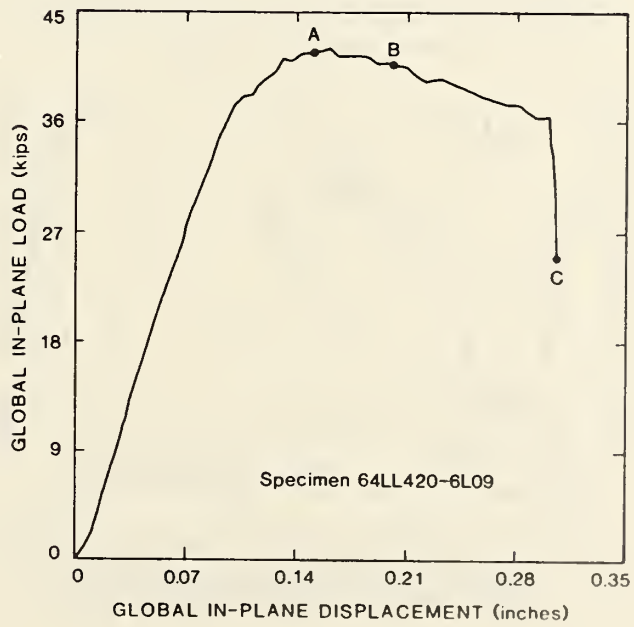
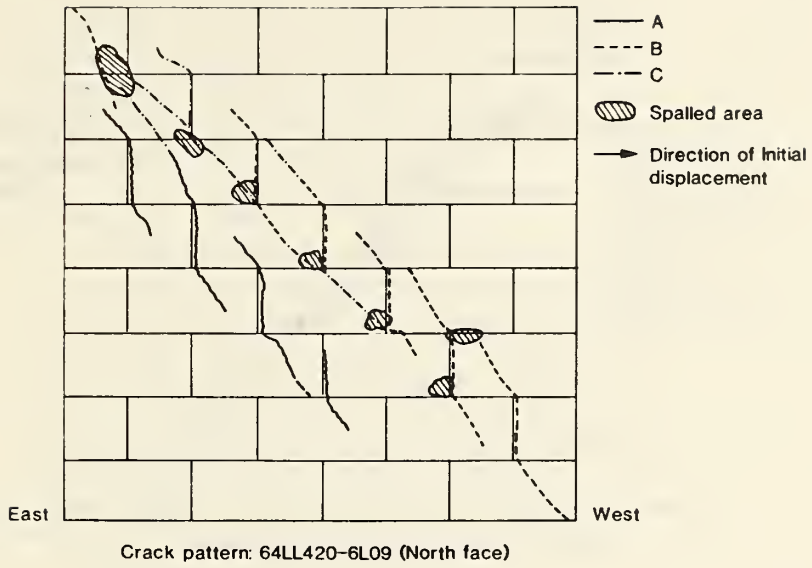


Figure E.8 Crack pattern and load-displacement curve for specimen 64LL420

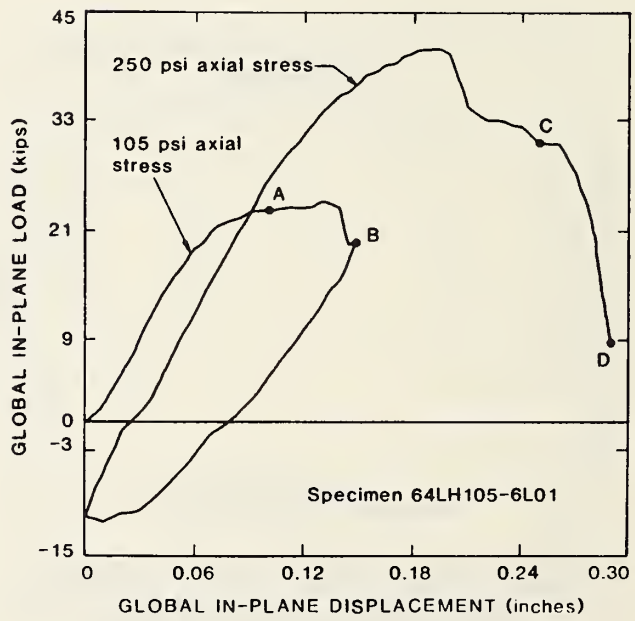
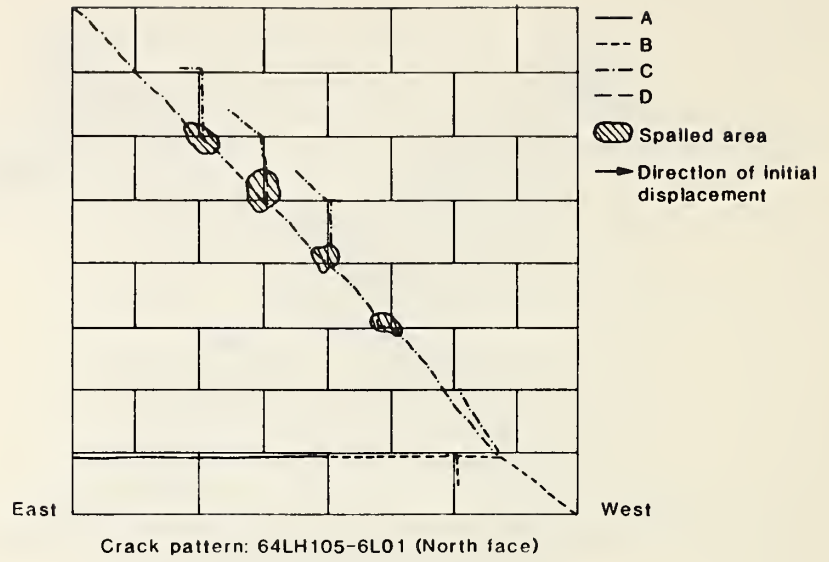


Figure E.9 Crack pattern and load-displacement curve for specimen 64LH105

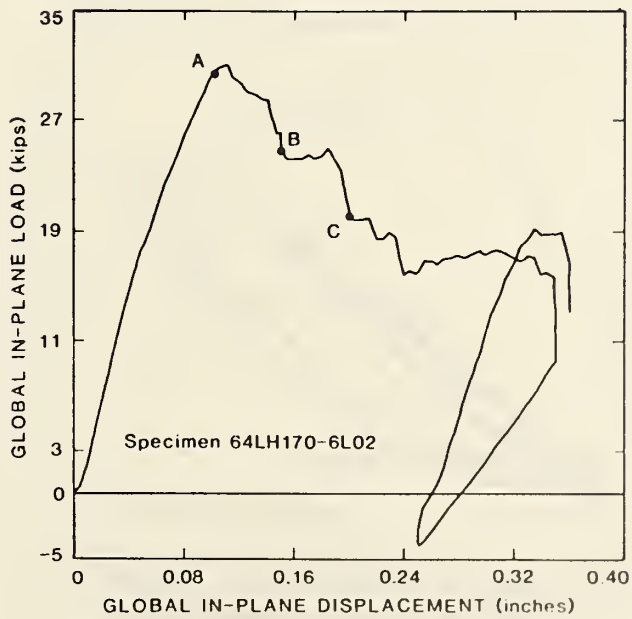
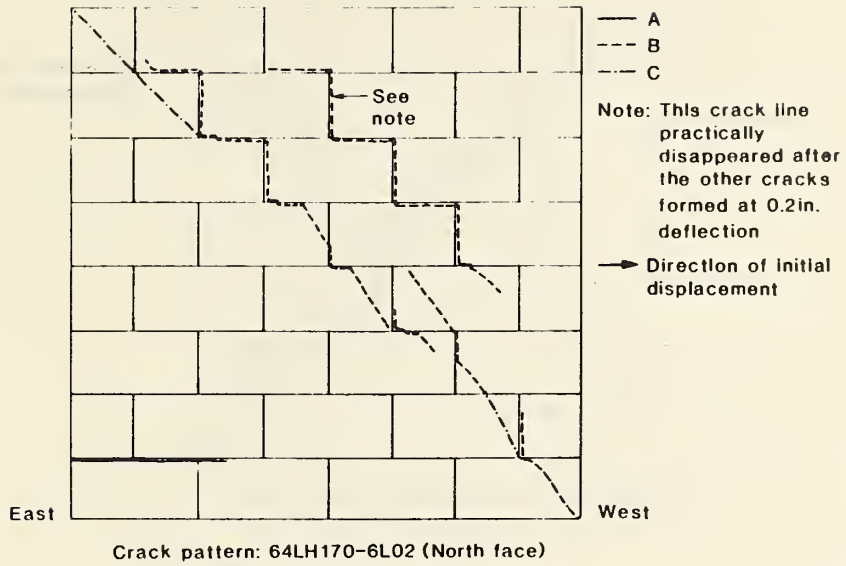


Figure E.10 Crack pattern and load-displacement curve for specimen 64LH170

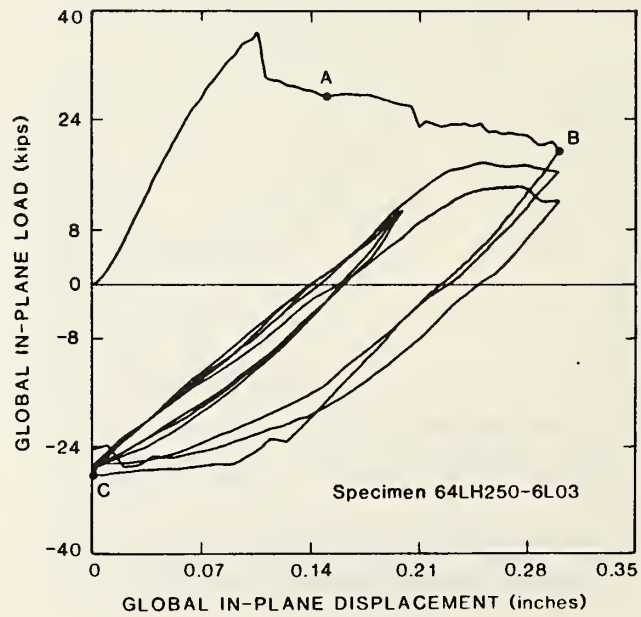
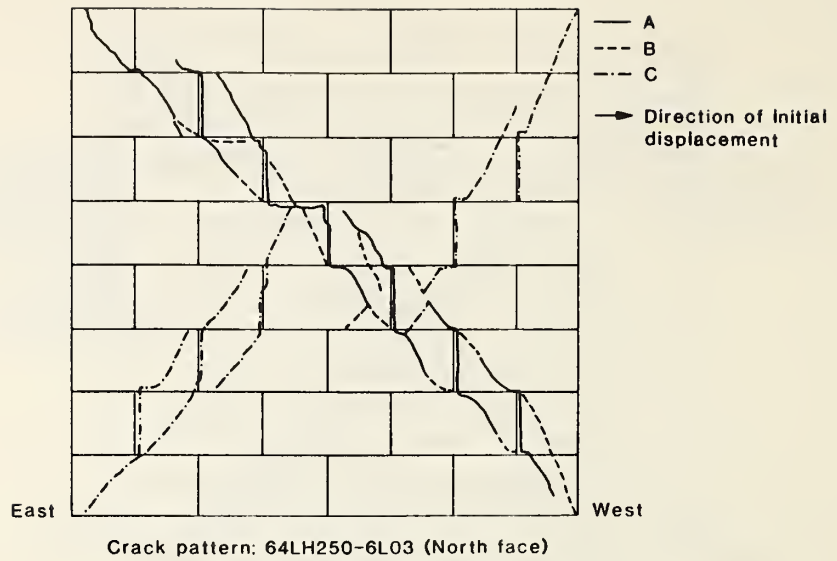


Figure E.11 Crack pattern and load-displacement curve for specimen 64LH250

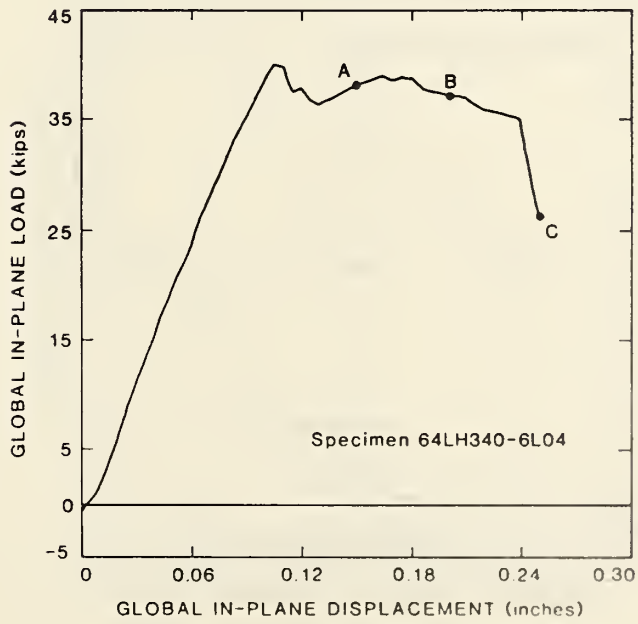
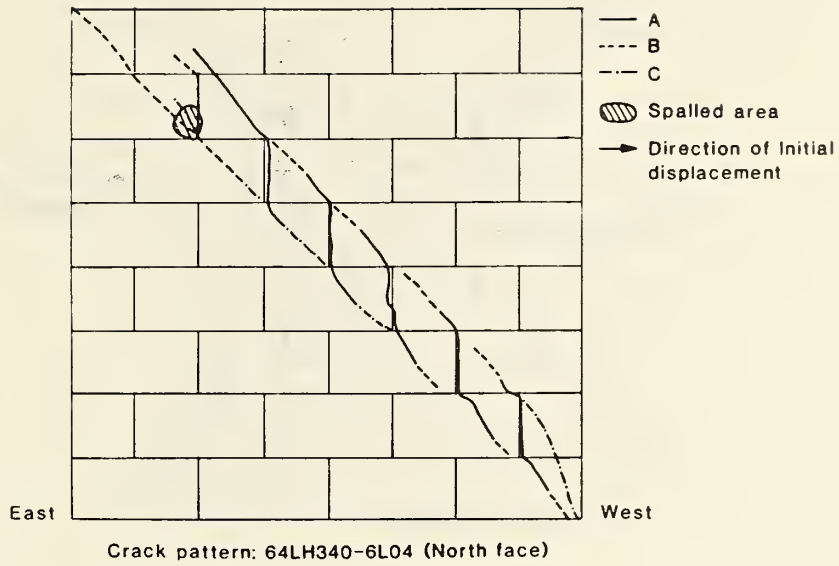


Figure E.12 Crack pattern and load-displacement curve for specimen 64LH340

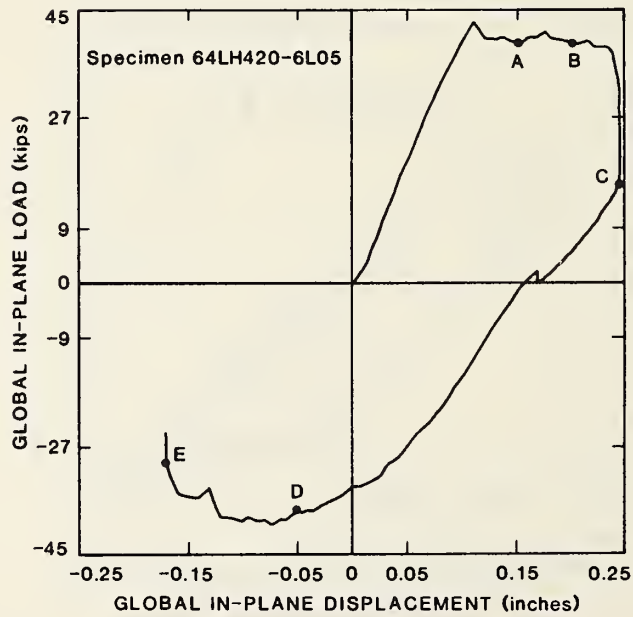
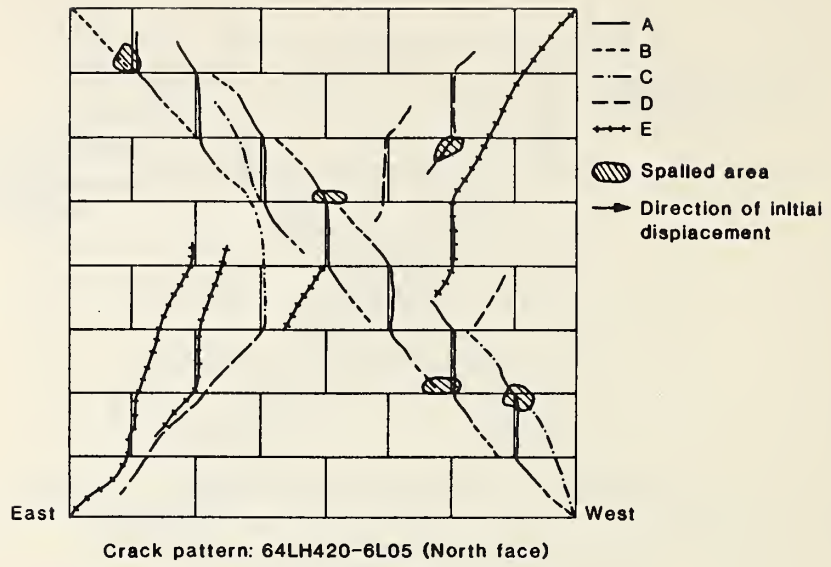


Figure E.13 Crack pattern and load-displacement curve for Specimen 64LH420

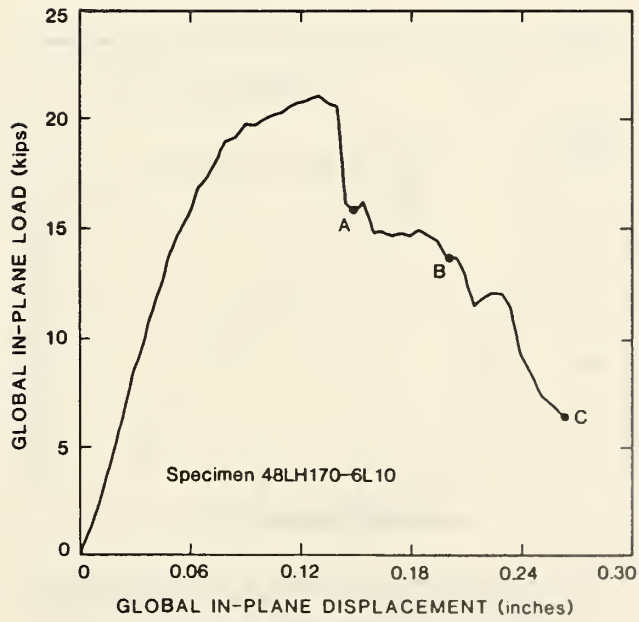
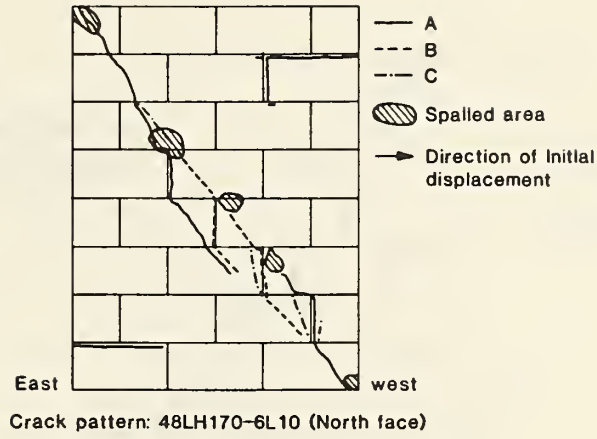


Figure E.14 Crack pattern and load-displacement curve for specimen 48LH170

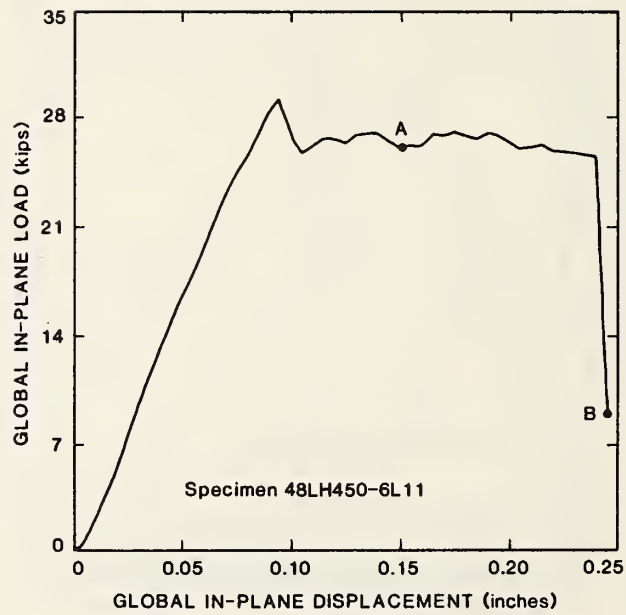
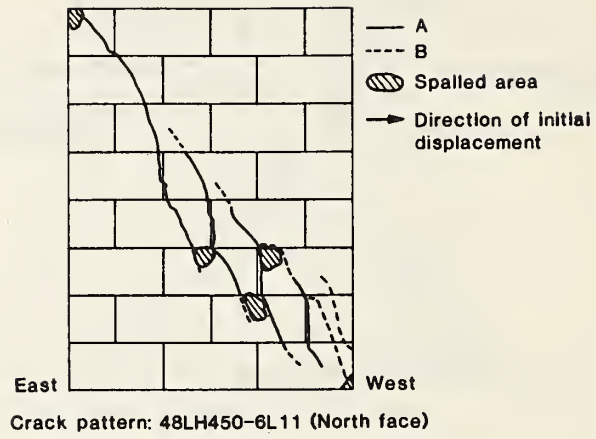


Figure E.15 Crack pattern and load-displacement curve for specimen 48LH450

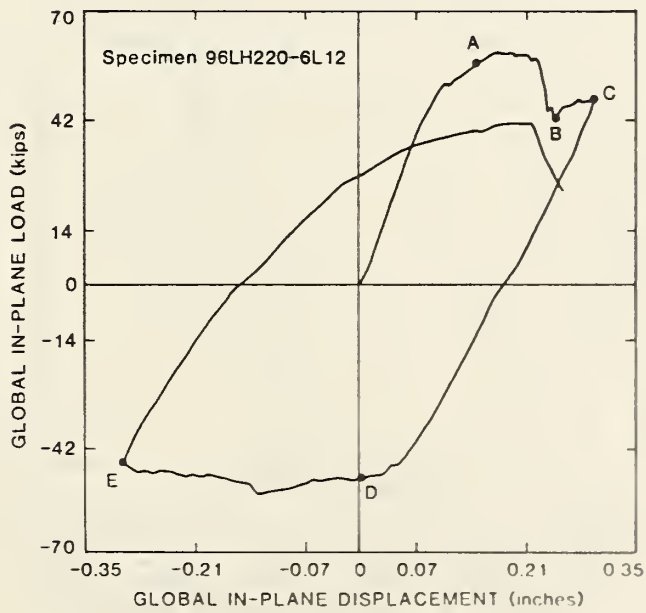
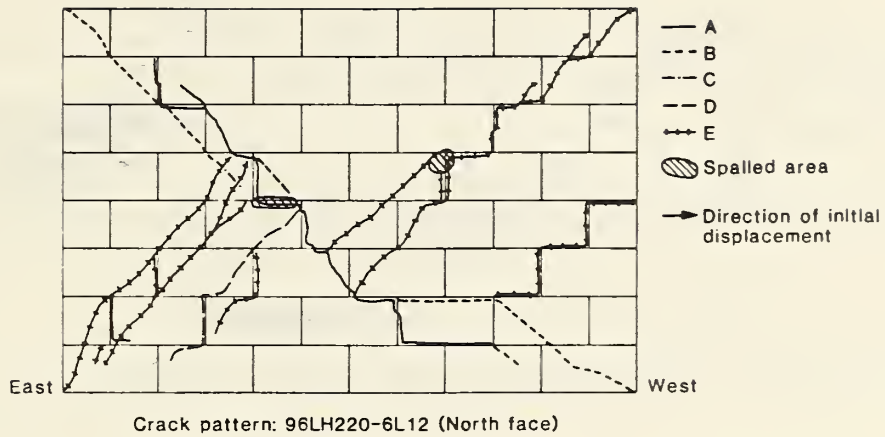


Figure E.16 Crack pattern and load-displacement curve for specimen 96LH220

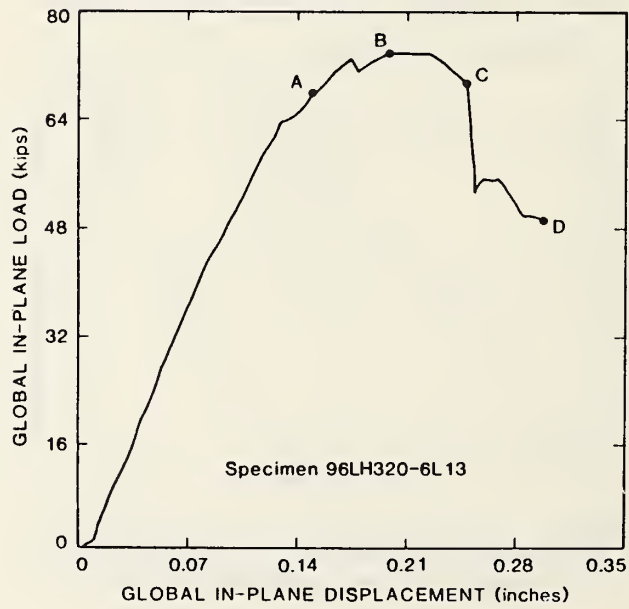
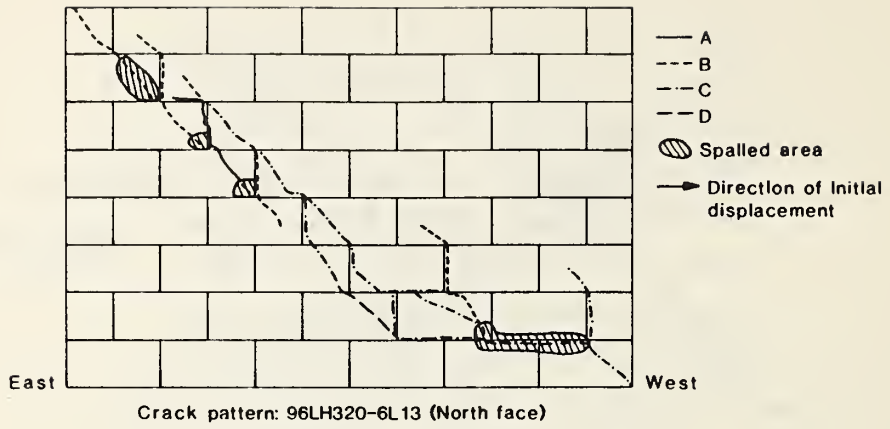


Figure E.17 Crack pattern and load-displacement curve for specimen 96LH320

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10. SUPPLEMENTARY NOTES <input type="checkbox"/> Document describes a computer program; SF-185, FIPS Software Summary, is attached.			
11. ABSTRACT <i>(A 200-word or less factual summary of most significant information. If document includes a significant bibliography or literature survey, mention it here)</i> Data from seventeen masonry wall panel tests are presented. All of the walls are ungrouted, unreinforced, and constructed with hollow concrete block. The primary variables in the test series are block and mortar strength, but the applied vertical compressive stress and wall aspect ratio are also varied. The walls are built with either a "high" strength block or a "low" strength block having gross area unit strengths of approximately 1800 psi and 1300 psi, respectively. The mortar is either a Type S or Type N mortar and, for convenience, is identified as high and low strength mortar, respectively. Thirteen of the wall panels have nominal dimensions of 64 in. long x 64 in. high x 8 in. thick, but two of the walls are 96 in. long and the remaining two walls are 48 in. long. The applied net area vertical compressive stress is constant for a given test, but varies between 100 psi and 400 psi for tests in the series reported herein. The walls are tested in the NBS Tri-directional Test Facility using fixed-ended boundary conditions at the top and bottom of the walls. A vertical compressive stress is applied and maintained while in-plane lateral displacements are imposed at the top of the wall. The test results indicate that for the lower levels of vertical compressive stress the effect of block and mortar strength on maximum in-plane shear resistance is relatively small, while with increased vertical compressive stress the effect of increasing material strength becomes significant and results in increased shear resistance. There is an interaction between the block and mortar strength such that the wall behavior may not be adequately predicted by considering the two strengths independently.			
12. KEY WORDS <i>(Six to twelve entries; alphabetical order; capitalize only proper names; and separate key words by semicolons)</i> Axial stress; block; concrete block; failure; masonry; mortar; shear; strength.			
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